

# UNIVERSITY OF MINNESOTA: TWIN CITIES

## CE 4511 HYDRAULIC STRUCTURES

### HW 5: Stilling Basins

CHRIS [REDACTED] LOGSTON

APRIL 13<sup>TH</sup>, 2015

#### Contents

<b>0 Preliminaries</b> . . . . .	<b>1</b>
0.1 Introduction . . . . .	1
0.2 Given Properties . . . . .	1
<b>1 Option 1: Simple Smooth Sloped Spillway</b> . . . . .	<b>2</b>
1.1 Crest Head Calculations . . . . .	2
1.2 Head Loss Calculation . . . . .	4
1.3 Stilling Basin Length Calculation . . . . .	6
1.4 Drop Length Check . . . . .	7
1.5 Basin Type . . . . .	7
1.6 Basin Feature Sizing . . . . .	8
1.7 Cost . . . . .	9
1.8 Tailwater - Conjugate Depth Plot . . . . .	10
<b>2 Option 2: Stepped Spillway</b> . . . . .	<b>11</b>
2.1 Testing Trial Parameters Procedure . . . . .	11
2.2 Trial Results . . . . .	13
2.3 Basin Feature Sizing . . . . .	14
2.4 Cost . . . . .	14
2.5 Tailwater - Conjugate Depth Plot . . . . .	15
<b>3 Option 3: Plunge Pool</b> . . . . .	<b>16</b>
3.1 Determination of Invert Elevation . . . . .	16
3.2 Trial Results . . . . .	17
3.3 Basin Feature Sizing . . . . .	18
3.4 Cost . . . . .	19
3.5 Tailwater - Conjugate Depth Plot . . . . .	20
<b>4 Option Choice</b> . . . . .	<b>21</b>
<b>A Appendix</b> . . . . .	<b>23</b>
A.1 Tailwater - Conjugate Depth Plots for All Options . . . . .	23
A.2 Charts and Figures Used . . . . .	24
A.3 Matlab <sup>®</sup> Scripts . . . . .	27

# 0) Preliminaries

## 0.1) Introduction

The client is a group of spillway managers. Due to extensive scouring, the group is burdened with continuous maintenance of the region immediately-downstream of the structure. Figure 0.1 shows the the spillway’s current configuration in which its end acts as a weir, the discharge across which leads to erosion. This firm proposes the addition of a post-spillway stilling basin in order to mitigate these scouring effects.

Considering both the volume of concrete and excavation required for such a project, three separate basin options are considered in order to determine the most cost effective development. The first, as discussed in section 1, is a smooth, sloped face ending in a simple concrete stilling basin. The second second option, as discussed in section 2, swaps the smooth, sloped spillway for one composed of a series of energy-dissipating steps. Finally, the third option in section 3 is analyzed as an abrupt drop structure, also known as a plunge pool.

Finally, as discussed in section 4, the optimal development is decided to be **the first option**.

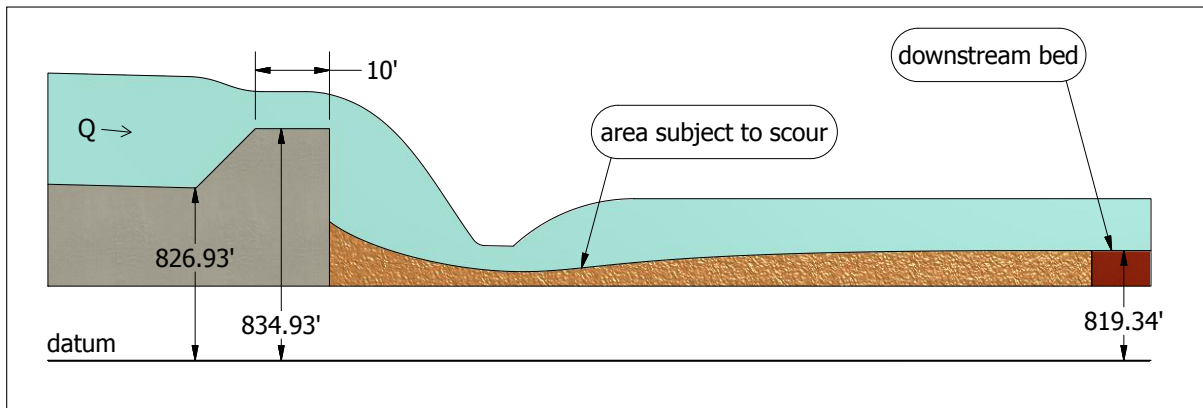


Figure 0.1: Current Scouring

## 0.2) Given Properties

The client’s current system dimensions in addition to that shown in figure 0.1 are as follows:

- upstream bed elevation = 826.93ft
- weir crest elevation:  $z_1 = 834.93ft$
- weir breadth:  $b = 10ft$
- downstream bed elevation = 819.34ft
- system width, perpendicular to flow:  $w = 160ft$

The client has also provided the discharges and tailwater elevations for several flood frequency events:

Flood Frequency (FF) (years)	Discharge (Q) ( $ft^3/s$ )	Tailwater Elevation ( $T_w$ ) (ft)
5	4,700	826.82
25	7,600	828.77
50	8,900	829.56
100	10,200	830.31
500	13,500	832.14

Table 0.1: Given Flood Frequency Properties

# 1) Option 1: Simple Smooth Sloped Spillway

A smooth, concrete spillway and stilling basin is the first system to be considered. As shown in figure 1.1, the end sill of the system currently in place will act as a weir after which a sloped surface of (2H:1V) will feed into a stilling basin. This basin will terminate in a riprap feeding into the downstream natural bed. While it is anticipated that the elevation of the basin will differ slightly from that of the downstream bed in order to contain the expected hydraulic jump, this difference is minimized in order to reduce excavation costs associated with this option.

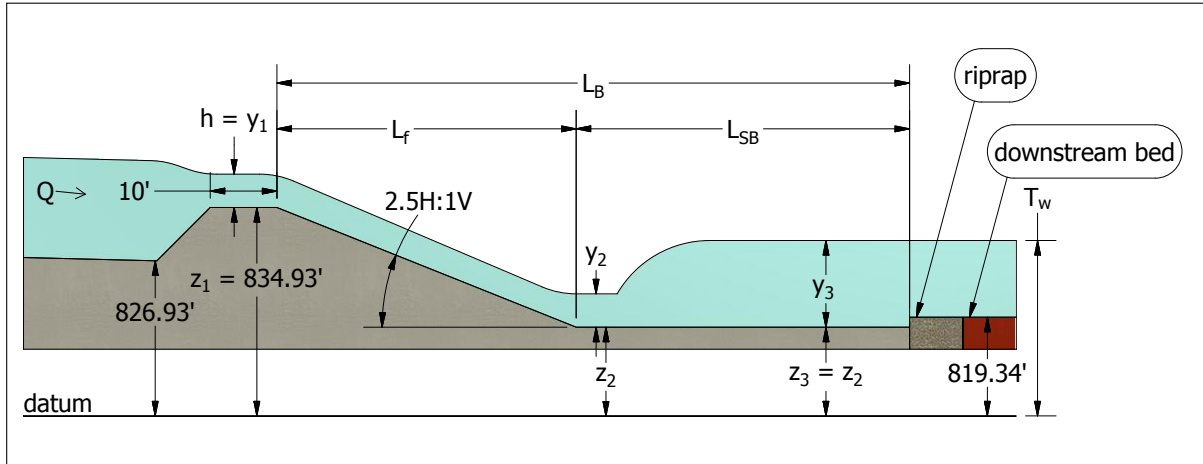


Figure 1.1: Option 1 Basin Detail

## 1.1) Crest Head Calculations

The formula for discharge over a broad crested weir is:

$$Q = \frac{2}{3} \sqrt{\frac{2}{3}g} (C_v C_d L_{\perp} H^{\frac{3}{2}}) \tag{1.1}$$

Where:

- $C_v$  is the coefficient accounting for kinetic effects, assumed at low velocities  $C_v \approx 1$ ;
- $C_d$  is the coefficient of discharge for a broad crested weir;
- $L_{\perp}$  is the system width perpendicular to flow;
- $H$  is total energy head at the crest;
- $g$  is the acceleration of gravity.

As shown in appendix figure A.4,  $C_d$  is a function of not only the crest head to breadth (b) ratio, but also the head to head-dam-height-sum ratio. That is:

$$C_d = f\left(\frac{h}{b}, \frac{h}{h+P}\right)$$

Therefore, although  $Q$  is known for each flood event,  $h$  cannot simply be isolated from eq. 1.1 and calculated, given the interdependency of  $C_d$ . Therefore, the crest head ( $h$ ) for each flood event is determined by using Matlab<sup>®</sup> solver to find a value of  $h$  for each  $Q$  in which the corresponding  $C_d$  matches both the  $\frac{h}{b}$  as well as the  $\frac{h}{h+P}$  parameters.

As shown in figure 1.2, this is achieved by interpolating between neighboring curves of known  $\frac{h}{h+P}$  values for any given  $\frac{h}{b}$  value. The heads determined through this process are shown in table 0.1.

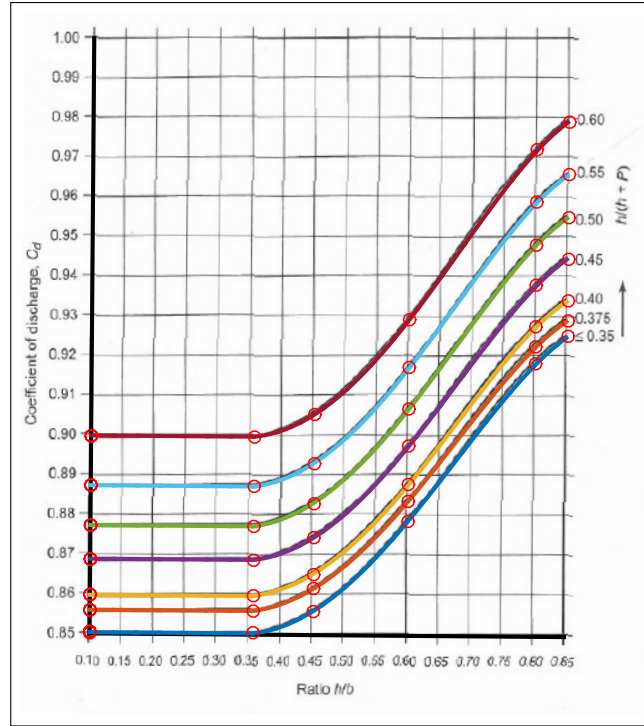


Figure 1.2: Discharge Coefficients Regression

Flood Frequency (years)	Discharge (Q) ( $ft^3/s$ )	Tailwater Elevation ( $T_w$ ) ( $ft$ )	Crest Head ( $h, y_1$ ) ( $ft$ )
5	4,700	826.82	4.935
25	7,600	828.77	6.586
50	8,900	829.56	7.221
100	10,200	830.31	7.818
500	13,500	832.14	9.298

Table 1.1: Given Flood Frequency Properties

## 1.2) Head Loss Calculation

In order to determine the hydraulic depth at the toe of the slope ( $y_2$ ), two equations for head loss, each a function of  $y_2$  are used. Head loss is quantified using the theories of energy balance ( $h_{l,eb}$ ) and boundary layers ( $h_{l,bl}$ ).

Starting with energy balance:

$$h_{l,eb} = y_1 + z_1 - y_2 - \frac{v_2^2}{2g} - z_2 \quad (1.2)$$

Where:

- $g$  is the acceleration of gravity

And continuing with boundary layers:

$$h_{l,bl} = \frac{\delta_3 U^3}{2gq} \quad (1.3)$$

Where:

- $\delta_3$  is a boundary distance from the sloped face calculated as:

$$\delta_3 = 0.22\delta \quad (1.4)$$

- $\delta$  is a further upstream boundary distance from the sloped face calculated as:

$$\delta = L_{hl}(0.02) \left( \frac{L_{hl}}{K_s} \right)^{-0.233} \quad (1.5)$$

- $L_{hl}$  is the length over the weir breadth and sloped face affecting head loss, approximated:

$$L_{hl} \approx b + \frac{z_1 - z_2}{\sin \alpha} \quad (1.6)$$

- $\alpha$  is the against-horizontal-angle of the slope, derived as:

$$\alpha = \tan^{-1} \left( \frac{1}{2.5} \right) \quad (1.7)$$

- $z_2$  is the stilling basin depth, as shown in figure 1.1.
- $K_s$  is the mean concrete roughness, provided by the client to be  $K_s = 0.002 ft$ .
- $q$  is the total discharge per system width, calculated:

$$q = \frac{Q}{w} \quad (1.8)$$

- $U$  is the velocity at the toe of the sloped surface, approximated:

$$U \approx v_2 = \frac{q}{y_2} \quad (1.9)$$

Given that minimal excavation is preferred, it may seem intuitive to solve for  $y_2$  by satisfying  $h_{l,eb} = h_{l,bl}$  with stilling basing bed set equal to natural downstream bed elevation ( $z_2 = 819.34ft$ ). However, in order to keep the post-slope hydraulic jump within the stilling basin, the conjugate depth of  $y_2$  (symbolized  $y_3$ ) must be checked such that the post-jump surface is no greater than the given tail-water elevation for each discharge. As shown in figure 1.1, this constraint is:

$$y_3 + z_2 < T_w \quad (1.10)$$

Where:

- $y_3$  is the conjugate depth of  $y_2$ , calculated:

$$y_3 = \frac{y_2}{2} [1 + 8Fr_2^2] \quad (1.11)$$

- $Fr_2$  is the Froude number at the toe of the slope, calculated:

$$Fr_2 = \frac{v_2}{\sqrt{gy_2^2}} = \frac{q}{y_2 \sqrt{gy_2^2}} \quad (1.12)$$

Therefore, the  $z_2$  at which the hydraulic jump is contained within the basin is determined by first starting at some arbitrary elevation for  $z_2$  at which the jump is well-contained within the basin (at the cost of excavation). This initial guess is set at  $10ft$  below the natural downstream bed elevation. After determining the corresponding  $y_2$  for this guessed  $z_2$ , the conjugate  $y_3$  is calculated. Then, the  $y_3 + z_2$  is compared to  $T_w$ , and the distance between those values is halved and added to the initially guessed  $z_2$  for a newly elevated, cheaper  $z_2'$ .

$$\Delta z_{2i} = \frac{1}{2} [T_w - (y_3 + z_{2i})] \quad (1.13)$$

This difference is used to create a new  $z_2$ :

$$z_{2i}' = z_{2i} + \Delta z_{2i} \quad (1.14)$$

A new  $y_2$  and  $h_l$  is calculated:

$$[y_{2i}, h_{li}] = f(z_{2i}')$$

The amount by which  $T_w$  exceeds  $z_2 + y_3$  is tested again, halved and fed back in the same series of steps. This process if carried out over many iterations until the value  $T_w - (y_3 + z_2)$  is below some safety threshold, chosen in this case to be  $0.2ft$ . A sample of this process is shown in table 1.2 for the 500 year flood event.

i	$z_2$ (ft)	$h_l$ (ft)	$y_2$ (ft)	$y_3$ (ft)	$T_w - (y_3 + z_2)$ (ft)	$z_2'$ (ft)
0	809.34	0.52023	1.84360	14.59300	8.20750	813.44
1	813.44	0.37877	1.97180	14.02200	4.67460	815.78
2	815.78	0.30878	2.05890	13.66200	2.69720	817.13
3	817.13	0.27183	2.11510	13.44000	1.57030	817.91
4	817.91	0.25147	2.15020	13.30600	0.91952	818.37
5	818.37	0.23993	2.17160	13.22500	0.54041	818.64
6	818.64	0.23328	2.18450	13.17700	0.31831	818.80
7	818.80	0.22942	2.19220	13.14800	0.18775	818.80

Table 1.2: Determination of  $z_2$  required for  $Q = 13,500 \frac{ft^3}{s}$

Table 1.3 presents the required  $z_2$  each of the five client-provided flooding events. Somewhat surprisingly, it is the 100 year flood, not the 500 year, that demands the lowest, most expensive  $z_2$ . Thus, this is set at the excavated  $z_2$  that is used for all flood event calculations for this first option.

FF (years)	Q (ft)	$z_2$ (ft)
5	4,700	819.1171
25	7,600	818.8963
50	8,900	818.8541
100	10,200	<b>818.8318</b>
500	13,500	818.9532

Table 1.3: Choice of safest  $z_2$

With  $z_2$  determined, table 1.4 presents the results of this process is carried out for each of the five client-provided discharge events.

FF (years)	Q (ft <sup>3</sup> /s)	q (ft <sup>2</sup> /s)	$y_2$ (ft)	$z_2$ (ft)	$h_{l,bl}$ (ft)	U, $v_2$ (ft/s)	$h_{l,eb}$ (ft)	Fr <sub>2</sub> -	$y_3$ (ft)	$y_3 + z_2$ (ft)	T <sub>w,elev</sub> (ft)
5	4,700	29.375	0.825	818.832	<b>0.521</b>	35.607	<b>0.521</b>	6.909	7.658	826.490	826.820
25	7,600	47.500	1.290	818.832	<b>0.356</b>	36.808	<b>0.356</b>	5.710	9.796	828.627	828.770
50	8,900	55.625	1.495	818.832	<b>0.314</b>	37.219	<b>0.314</b>	5.365	10.617	829.449	829.560
100	10,200	63.750	1.696	818.832	<b>0.283</b>	37.587	<b>0.283</b>	5.086	11.381	830.213	830.310
500	13,500	84.375	2.194	818.832	<b>0.229</b>	38.464	<b>0.229</b>	4.577	13.143	831.975	832.140

Table 1.4: Results of  $h_l$ , and  $y_2$  for each flood event event

### 1.3) Stilling Basin Length Calculation

Appendix chart A.5 presents recommended ratios of total stilling basin length per toe depth ( $y_2$ ) for various toe Froude numbers ( $Fr_2$ ). Thus, with known values of  $Fr_2$ , these ratios are read from the type III curve as:

$$\frac{L_{SB}}{y_3} = f(Fr_2) \quad (1.15)$$

These basin lengths are then calculated as the product this ratio and known  $y_2$ :

$$L_{SB} = \frac{L_{SB}}{y_3} y_3 \quad (1.16)$$

Table 1.5 presents the results of this process, added to previous tabulation, for these basin lengths.

FF (years)	Q (ft <sup>3</sup> /s)	Fr <sub>2</sub> -	$\frac{L_{SB}}{y_2}$ -	L <sub>SB</sub> (ft)
5	4,700	6.909	2.543	<b>19.471</b>
25	7,600	5.710	2.415	<b>23.657</b>
50	8,900	5.365	2.368	<b>25.146</b>
100	10,200	5.086	2.327	<b>26.484</b>
500	13,500	4.577	2.242	<b>29.466</b>

Table 1.5: Basin length results

Thus, the built total horizontal basin length will be the greatest required for this basin. Reading from table 1.5, this is a basin length of:

$$L_{SB_{opt1}} = 29.466 \text{ ft}$$

Also, the flow conditions ( $y_2, y_3, Fr_2$ ), for the flood event that determined this  $L_B$  will be used for feature sizing (if any) in section 1.6.

It should be noted that this estimate assumes the hydraulic jump starts immediately at the toe of the sloped face.

## 1.4) Drop Length Check

In order to make sure the previous head loss calculations are valid, the drop length ( $L_d$ ) of the system before development must be calculated and compared to the horizontal footprint the sloped face would have ( $L_f$ ).

Thus, the horizontal footprint of the slope is calculated as follows:

$$L_f = 2.5(z_1 - z_2) = 2.5(834.93 \text{ ft} - 818.83 \text{ ft}) = 40.25 \text{ ft}$$

The drop length is then calculated using:

$$L_d = 4.3(z_1 - z_2)D_r^{-0.27} \quad (1.17)$$

Where the drop number  $D_r$  is calculated as:

$$D_r = \frac{q^2}{g(z_1 - z_2)^3} \quad (1.18)$$

Table 1.6 presents the drop lengths for each flow calculated with the same  $z_2$  using equations 1.17 and 1.18.

FF	Q	Dr	L <sub>d</sub>
5	4,700	0.00642	17.715
25	7,600	0.01680	22.964
50	8,900	0.02303	25.008
100	1,0200	0.03025	26.919
500	13,500	0.05300	31.318

Table 1.6: Drop lengths against  $z_2$

Given that each calculated  $L_d$  is far less than  $L_f$ , it is assured that the post-weir flow will fall upon the sloped face, rendering the related head loss equations valid.

## 1.5) Basin Type

Table 1.7 presents recommendations of basin types for certain flow conditions, sourced from the USBR 1984 publication “Hydraulic Design of Stilling Basins and Energy Dissipators”.

Type	Conditions	Features
I	weak jumps, $Fr_2 < 2.5$	no additional structures needed
IV	$2.5 < Fr_2 < 4.5$	chute blocks, optional solid end sill
III	$Fr_2 > 4.5$ , $V_2 < 60 \text{ ft/s}$	chute blocks, baffle blocks, solid end sill
II	$Fr_2 > 4.5$ , $V_2 > 60 \text{ ft/s}$	chute blocks, no baffle blocks, dentated end sill

Table 1.7: Recommended basin types for different flow conditions

Thus, regarding table 1.4, where the lowest  $Fr_2$  is above 4.5 and the highest  $v_2$  is below  $60 \text{ ft/s}$ , a **type III** basin is chosen.



## 1.6) Basin Feature Sizing

Figure 1.3 from the same USBR 1984 article presents the recommended feature sizing for a type III basin. In this drawing, the dimensions  $D_1$  and  $D_2$  are equivalent to this report's  $y_2$  and  $y_3$ , respectively.

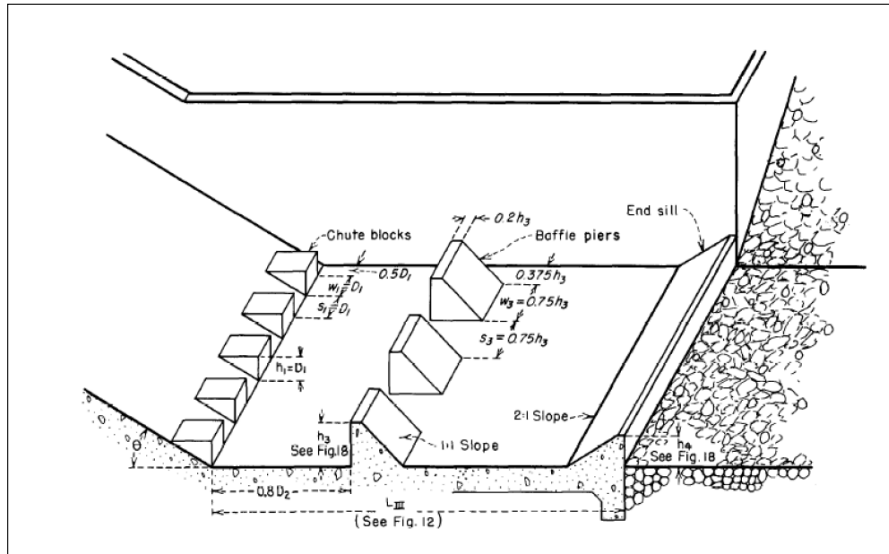


Figure 1.3: Feature detail for type III basin (USBR)

The dimensions for  $h_3$  and  $h_4$  are read from the following USBR chart:

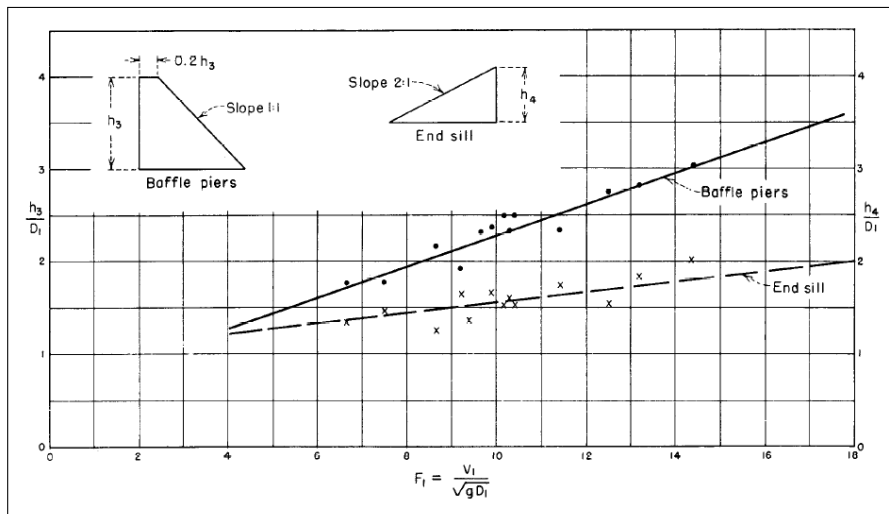


Figure 1.4: Chart for  $h_3/D_1$  and  $h_4/D_1$  as a function of  $Fr_2$

Thus, with  $Fr_2 = 4.5767$  for the 500 year event,  $h_3$  and  $h_4$  are read as follows:

- $h_3 = 3.0481 \text{ ft}$
- $h_4 = 2.7339 \text{ ft}$

Table 1.8 presents the dimensions determined for all featured recommended for a type III basin.

Description	Symbol	Method of Determination	Value (ft)
chute block height	$h_1$	$= D_1 = y_2$	2.194
chute block spacing	$s_1$	$s_1 = D_1 = y_2$	2.194
chute block width	$w_1$	$w_1 = D_1 = y_2$	2.194
chute block – wall gap	-	$= 0.5(D1) = 0.5(y_2)$	1.097
toe - upstream baffle face distance	-	$= 0.8(D2) = 0.8(y_3)$	10.515
baffle pier upstream face height	$h_3$	read from figure 1.4	3.048
end sill height above downstream elevation	$h_4$	read from figure 1.4	2.734
baffle pier – wall gap	-	$= 0.375(h_3)$	1.143
baffle pier top breadth	-	$= 0.2(h_3)$	0.610
baffle pier width	$w_3$	$w_3 = 0.75(h_3)$	2.286
baffle pier spacing	$s_3$	$s_3 = 0.75(h_3)$	2.286

Table 1.8: Option 1, type III basin feature sizes

### 1.7) Cost

The costs associated with the construction of this spillway are considered to be mainly from price for concrete volume as well as price of excavation from natural downstream bed elevation as well as the additional depth required to fill in the basin base, assumed to be  $t = 4 \text{ ft}$  thick.

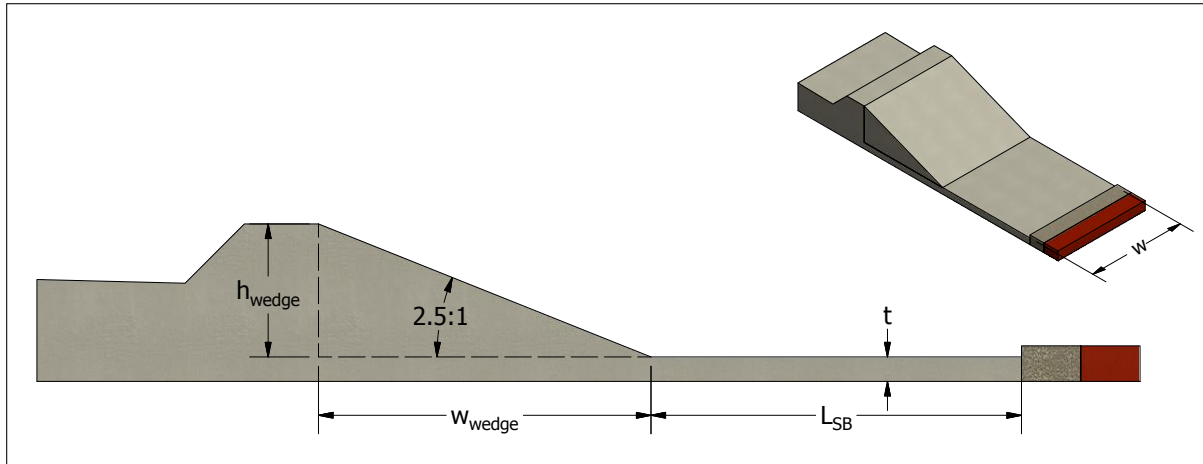


Figure 1.5: Option 1 concrete cost detail

Using the dimensions represented in figure 1.5, the total volume of concrete and excavation is calculated:

$$\begin{aligned}
 V_{\text{concrete}} &= V_{\text{sloped wedge}} + V_{\text{basin}} = \frac{1}{2} (b_{\text{wedge}})(h_{\text{wedge}}) w + t (L_{\text{SB}}) w \\
 &= \frac{1}{2} (z_1 - z_2)^2 (2.5) w + t (L_B) w \\
 &= 51,830 \text{ ft}^3 + 18,858 \text{ ft}^3 = \boxed{70,689 \text{ ft}^3}
 \end{aligned}$$

$$V_{\text{excavation}} = [t + (\text{natural downstream } z) - z_2] (L_B)(w) = \boxed{21,254 \text{ ft}^3}$$

### 1.8) Tailwater - Conjugate Depth Plot

As shown in figure 1.6, the post-hydraulic-jump depth of  $y_3$  does not cause the water surface to rise above tailwater elevation.

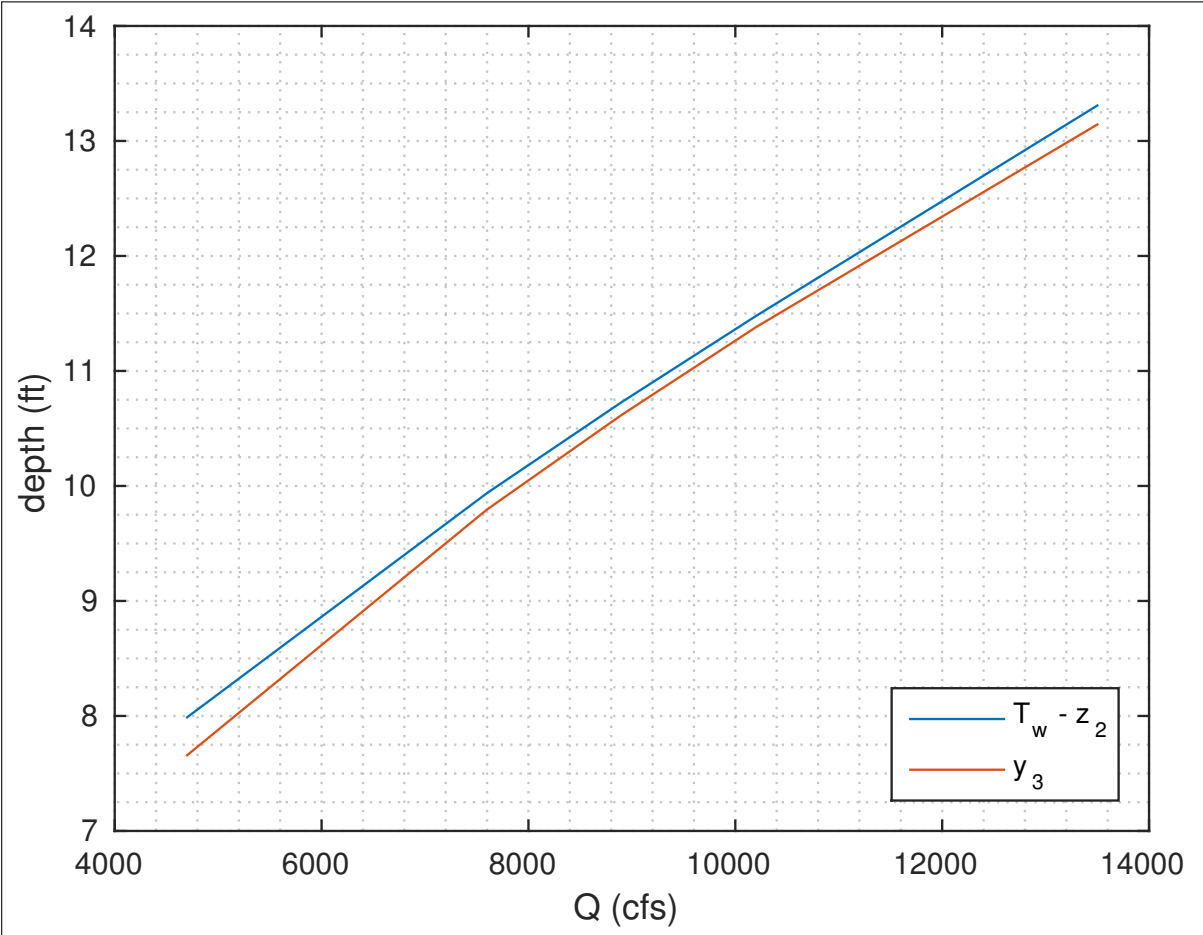


Figure 1.6: Option 1 plot of tailwater and conjugate depths

## 2) Option 2: Stepped Spillway

The second option under consideration is one involving a stepped spillway.

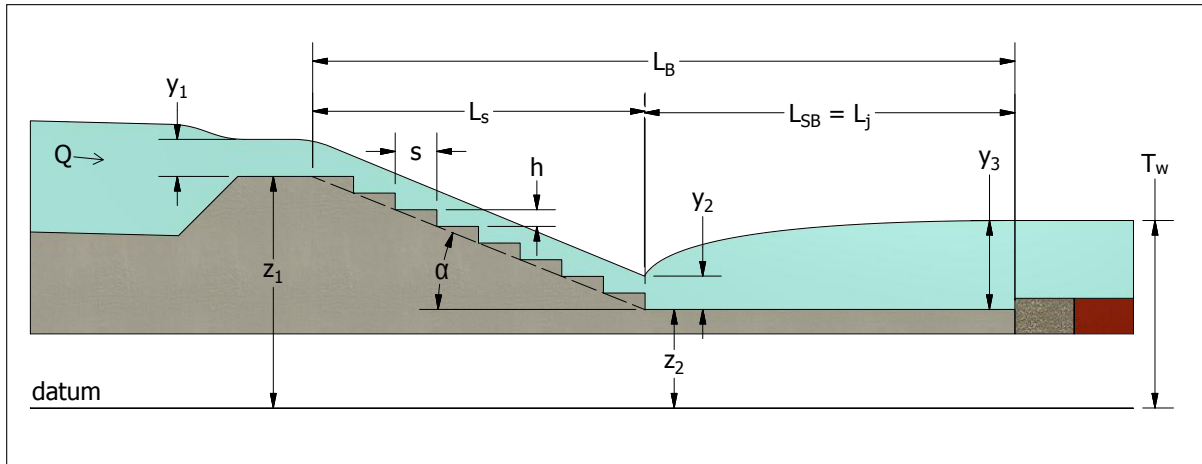


Figure 2.1: Option 2 basin detail

### 2.1) Testing Trial Parameters Procedure

In determining the most economically efficient construction of such a system, a system can be built that meets the following conditions:

- The hydraulic jump is contained within the basin
- The natural drop distance  $L_d$  is less than the horizontal footprint of the stepped spillway  $L_s$ , ensuring steps dissipate hydraulic energy via contact with water
- No transitional flow along steps (either entirely skimming or nappe flow)
  - This constraint ensures, no transition between nappe and skimming flow across the range of anticipated discharges.
  - Allowing for transitional flow incurs the risk of significant structural vibration, hence compromising the structural integrity of the entire system.

This is done by developing a system that presents relevant condition tests as a function of stepped spillway parameters  $s$ ,  $h$  and  $N$ . This is accomplished as follows:

- A reasonable step width ( $s$ ) and height ( $h$ ) are chosen.
- With this known ( $h$ ), an integer number of steps ( $N$ ) are chosen such that the total vertical distance covered by the steps is at least the distance between the crest elevation, ( $z_1$ ) and the natural downstream bed elevation.
- With this known  $N$ , the excavated  $z_2$  can be determined easily:

$$z_2 = z_1 - Ns \quad (2.1)$$

- The status of nappe flow vs. skimming is tested as follows:

$$\text{nappe for } \frac{y_c}{h} \leq 0.89 - 0.4 \left( \frac{h}{s} \right), \quad \text{skimming for } \frac{y_c}{h} \geq 1.2 - 0.325 \left( \frac{h}{s} \right) \quad (2.2)$$

- where  $y_c$  can be solved:

$$y_c = \left( \frac{q^2}{g} \right)^{1/3} \quad (2.3)$$

- It is important to maintain only either nappe flow or skimming flow for all discharges. That way, detrimental transitional flow can be avoided for all anticipated discharge.

- The depth at the toe ( $y_2$ ) is then calculated:

$$y_2 = y_c \left[ \frac{f_c}{8 \sin(\alpha)} \right]^{1/3} \quad (2.4)$$

- where  $f_c$ , the coefficient, of friction is calculated:

$$f_c = \frac{f_d}{2} \left[ 1 + \tanh \left( 2.5 \frac{0.5 - c_{\text{mean}}}{c_{\text{mean}}(1 - c_{\text{mean}})} \right) \right] \quad (2.5)$$

- where  $f_d$ , another coefficient, is calculated:

$$f_d = \frac{2}{\sqrt{\pi}} \frac{1}{k} \quad \text{where } k = 4.5 \quad (2.6)$$

- where  $c_{\text{mean}}$ , the mean air concentration, is calculated:

$$c_{\text{mean}} = 1.44 \sin(\alpha) - 0.08 \quad (2.7)$$

- and where  $\alpha$ , the pseudo-slope angle, is calculated:

$$\alpha = \tan^{-1} \left( \frac{h}{s} \right) \quad (2.8)$$

- with this  $y_2$  known, the toe velocity ( $v_2$ ) is calculated:

$$v_2 = \frac{q}{y_2} \quad (2.9)$$

- which allows the toe Froude number ( $Fr_2$ ) to be calculated:

$$Fr_2 = \frac{v_2}{\sqrt{(g)(y_2)}} \quad (2.10)$$

- At this point, the known  $v_2$  and  $Fr_2$  can be used to determine basin type using table 1.7.  $Fr_2$  can also be used to determine the conjugate depth ( $y_3$ ) using equation 1.11.

- This known  $y_3$  can then be compared to the tailwater depth to ensure the jump is contained within the basin.

- As a final step, it is necessary to check that the drop length  $L_d$  is less than the stepped spillway horizontal footprint ( $L_s$ ). This is accomplished using eqs. 1.17 and 1.18 from the option 1 process.

## 2.2) Trial Results

Table 2.1 presents the results of this procedure carried out for the parameters  $h = 1$ ,  $s = 2.1$ ,  $N = 16$ . As shown, the following conditions are met for all discharges:

- Flow is constantly skim throughout given that all  $y_c/h$  exceeds  $(y_c/h)_{\text{nappe}}$
- Any change in  $Fr_2$  does not indicate transitional flow across changing discharges
- The natural drop length falls below  $L_S$ ;

However, the values of  $\frac{T_w - z_2}{y_3}$  for the 25 though 500 year floods indicate that the jump is in fact *not* held within the basin. Thus, the parameters are adjusted and tried again.

FF (yrs)	Q ( $f^3/s$ )	q ( $f^2/s$ )	$y_c$ ( $f^2/s$ )	$y_c/h$ -	$(y_c/h)_{\text{nappe}}$ -	$(y_c/h)_{\text{skim}}$ -	$y_2$ (ft)	$v_2$ ( $f/s$ )	$Fr_2$ -	$y_3$ (ft)	$T_w - z_2$ (ft)	$\frac{T_w - z_2}{y_3}$ -	$\frac{L_{SB}}{y_3}$ -	$L_{SB}$ (ft)	Dr -	$L_d$ (ft)	$\frac{L_d}{L_s}$ -
5	4,700	29.375	2.993	2.993			0.849	34.614	6.622	7.534	7.890	1.047	2.516	18.959	0.007	17.694	0.527
25	7,600	47.500	4.123	4.123			1.169	40.628	6.622	10.379	9.840	0.948	2.516	26.119	0.017	22.937	0.683
50	8,900	55.625	4.580	4.580	0.700	1.045	1.299	42.824	6.622	11.532	10.630	0.922	2.516	29.018	0.023	24.979	0.743
100	10,200	63.750	5.016	5.016			1.423	44.815	6.622	12.629	11.380	0.901	2.516	31.779	0.031	26.887	0.800
500	13,500	84.375	6.047	6.047			1.715	49.204	6.622	15.224	13.210	0.868	2.516	38.309	0.054	31.281	0.931

Table 2.1: Results for trial with  $h = 1$ ,  $s = 2.1$ ,  $N = 16$

For a second round, the step dimensions are kept, but an additional step is added ( $N = 17$ ), requiring an even lower  $z_2$ . However, as shown in table 2.2, this still does not contain the hydraulic jump for the 100 and 500 year discharges. Thus, the step dimension  $s$  is modified, and the process is repeated.

FF (yrs)	Q ( $f^3/s$ )	q ( $f^2/s$ )	$y_c$ ( $f^2/s$ )	$y_c/h$ -	$(y_c/h)_{\text{nappe}}$ -	$(y_c/h)_{\text{skim}}$ -	$y_2$ (ft)	$v_2$ ( $f/s$ )	$Fr_2$ -	$y_3$ (ft)	$T_w - z_2$ (ft)	$\frac{T_w - z_2}{y_3}$ -	$\frac{L_{SB}}{y_3}$ -	$L_{SB}$ (ft)	Dr -	$L_d$ (ft)	$\frac{L_d}{L_s}$ -
5	4,700	29.375	2.993	2.993			0.849	34.614	6.622	7.534	8.890	1.180	2.516	18.959	0.005	17.899	0.501
25	7,600	47.500	4.123	4.123			1.169	40.628	6.622	10.379	10.840	1.044	2.516	26.119	0.014	23.203	0.650
50	8,900	55.625	4.580	4.580	0.700	1.045	1.299	42.824	6.622	11.532	11.630	1.009	2.516	29.018	0.020	25.268	0.708
100	10,200	63.750	5.016	5.016			1.423	44.815	6.622	12.629	12.380	0.980	2.516	31.779	0.026	27.199	0.762
500	13,500	84.375	6.047	6.047			1.715	49.204	6.622	15.224	14.210	0.933	2.516	38.309	0.045	31.644	0.886

Table 2.2: Results for trial with  $h = 1$ ,  $s = 2.1$ ,  $N = 17$

With a new step horizontal length ( $s$ ), all conditions are met, as shown in table 2.3. All hydraulic jumps are contained, flow is skimming throughout, no transitional flow is detected, and all drop lengths fall below  $L_S$ .

FF (yrs)	Q ( $f^3/s$ )	q ( $f^2/s$ )	$y_c$ ( $f^2/s$ )	$y_c/h$ -	$(y_c/h)_{\text{nappe}}$ -	$(y_c/h)_{\text{skim}}$ -	$y_2$ (ft)	$v_2$ ( $f/s$ )	$Fr_2$ -	$y_3$ (ft)	$T_w - z_2$ (ft)	$\frac{T_w - z_2}{y_3}$ -	$\frac{L_{SB}}{y_3}$ -	$L_{SB}$ (ft)	Dr -	$L_d$ (ft)	$\frac{L_d}{L_s}$ -
5	4,700	29.375	2.993	2.993			1.037	28.331	4.903	6.690	8.890	1.329	2.298	15.373	0.005	17.899	0.458
25	7,600	47.500	4.123	4.123			1.428	33.253	4.903	9.216	10.840	1.176	2.298	21.178	0.014	23.203	0.593
50	8,900	55.625	4.580	4.580	0.716	1.059	1.587	35.050	4.903	10.240	11.630	1.136	2.298	23.529	0.020	25.268	0.646
100	10,200	63.750	5.016	5.016			1.738	36.680	4.903	11.214	12.380	1.104	2.298	25.768	0.026	27.199	0.696
500	13,500	84.375	6.047	6.047			2.095	40.272	4.903	13.518	14.210	1.051	2.298	31.063	0.045	31.644	0.809

Table 2.3: Results for trial with  $h = 1$ ,  $s = 2.3$ ,  $N = 17$

Thus the parameters set for further investigation are  $h = 1$ ,  $s = 2.3$ ,  $N = 17$ . The recommended basin for the corresponding values of  $Fr_2$  and  $v_2$  for all discharges is again type III. Also, given that the 500 year flood requires the largest basin, its conditions will be used for all feature sizing, and this required length will be the design basin length.

$$L_{SB_{opt2}} = 31.063 \text{ ft}$$

## 2.3) Basin Feature Sizing

Table 2.4 presents the dimensions determined for all features recommended for a type III basin in this case.

Description	Symbol	Method of Determination	Value (ft)
chute block height	$h_1$	$= D_1 = y_2$	2.095
chute block spacing	$s_1$	$s_1 = D_1 = y_2$	2.095
chute block width	$w_1$	$w_1 = D_1 = y_2$	2.095
chute block – wall gap	-	$= 0.5(D_1) = 0.5(y_2)$	1.048
toe - upstream baffle face distance	-	$= 0.8(D_2) = 0.8(y_3)$	10.814
baffle pier upstream face height	$h_3$	read from figure 1.4	3.027
end sill height above downstream elevation	$h_4$	read from figure 1.4	2.651
baffle pier – wall gap	-	$= 0.375(h_3)$	1.135
baffle pier top breadth	-	$= 0.2(h_3)$	0.605
baffle pier width	$w_3$	$w_3 = 0.75(h_3)$	2.270
baffle pier spacing	$s_3$	$s_3 = 0.75(h_3)$	2.270

Table 2.4: Option 2, type III basin feature sizes

## 2.4) Cost

Just as with option 1, the primary costs of this stepped spillway system are both cost of concrete as well as excavation, both measured in volume. Stilling basin slab thickness ( $t$ ) is again set to 4 ft.

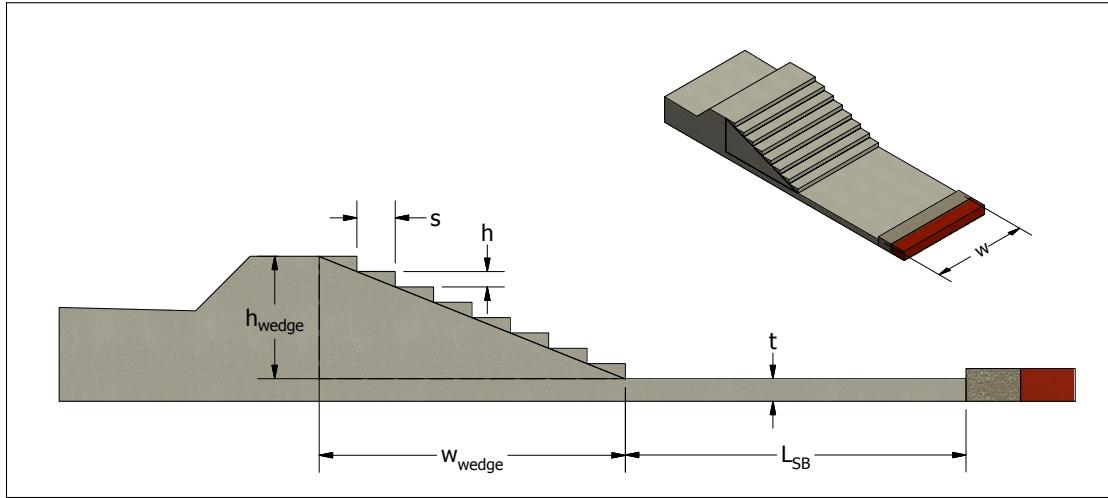


Figure 2.2: Option 2 concrete cost detail

Using the dimensions represented in figure 2.2, the total volume of concrete and excavation is calculated:

$$\begin{aligned}
 V_{\text{concrete}} &= V_{\text{sloped wedge}} + V_{\text{steps}} + V_{\text{basin}} = \frac{1}{2} (w_{\text{wedge}}) (h_{\text{wedge}}) w + \frac{w}{2} (s)(h)w + (t) (L_B) w \\
 &= \frac{w}{2} (Ns)(Nh) + \frac{w}{2} (sh)(N) + t(w)(L_{SB}) = 53,176 \text{ ft}^3 + 31,280 \text{ ft}^3 + 19,880 \text{ ft}^3 = \boxed{76,184 \text{ ft}^3}
 \end{aligned}$$

$$V_{\text{excavation}} = [t + (\text{natural downstream } z - z_2)] (L_{SB}) (w) = \boxed{26,888 \text{ ft}^3}$$

## 2.5) Tailwater - Conjugate Depth Plot

As shown in figure 2.3, the post-hydraulic-jump depth of  $y_3$  does not cause the water surface to rise above tailwater elevation for this appropriately sized stepped spillway.

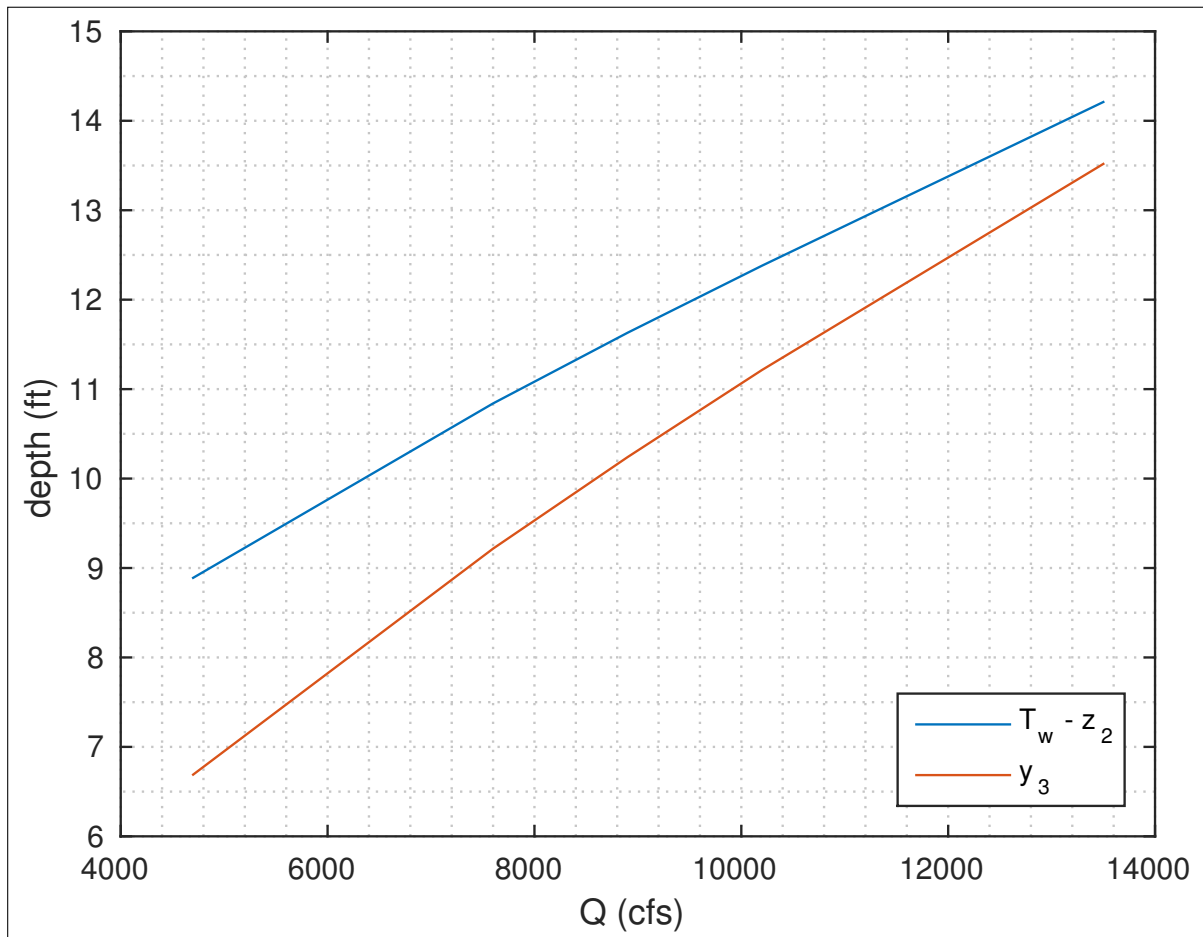


Figure 2.3: Option 2 plot of tailwater and conjugate depths



### 3) Option 3: Plunge Pool

The final option to be considered is simple drop structure (or plunge pool), over which energy is dissipated via impact with the basin bed as well as the end sill.

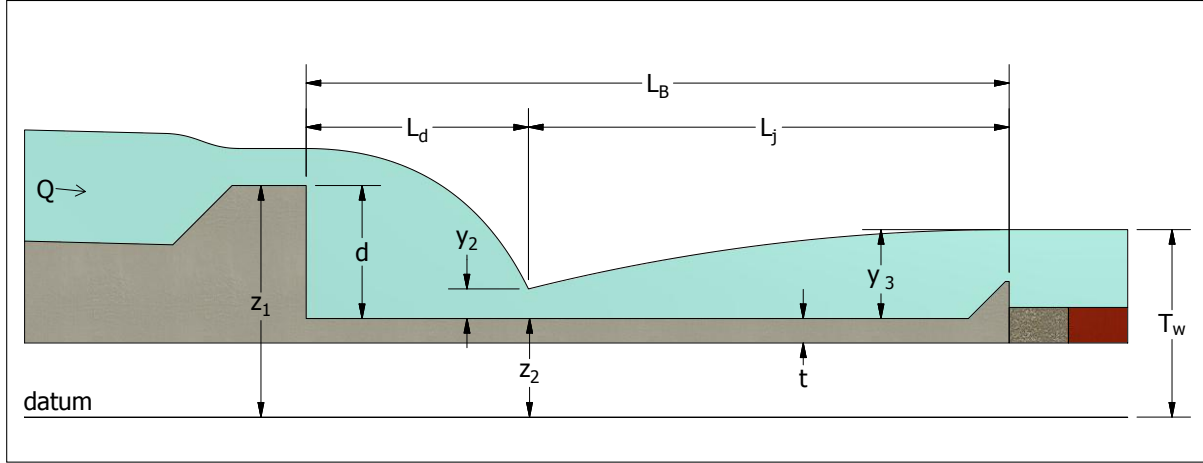


Figure 3.1: Option 3 Basin Detail

As shown in figure 3.1, the system is sized such that the hydraulic jump starts immediately at the point at the drop length ( $L_d$ ). As with previous options, the relation between  $y_3$  and  $T_w$  is primarily a function of chosen  $z_2$ .

#### 3.1) Determination of Invert Elevation

USBR recommends differing safety offsets of  $T_w - (y_3 + z_2)$  for differing basins. Therefore, an initial  $z_2$  is determined in which this offset is 0. Then, the basin type can be inferred from available conditions and a new  $z_2$  can be set matching the corresponding recommended offset value. This process is carried out as follows:

- $z_2$  is first set to natural downstream  $z$
- This invert elevation is used to calculate the drop height ( $d$ ) of the structure:

$$d = (z_1 - z_2) \quad (3.1)$$

- With this  $d$ , the drop number ( $Dr$ ) is calculated using equation 1.18.

$$Dr = \frac{q^2}{g(z_1 - z_2)^3} = \frac{q^2}{gd^3}$$

- The drop length hydraulic depth ( $y_2$ ) is then calculated:

$$y_2 = d(0.54)Dr^{0.425} \quad (3.2)$$

- Critical depth ( $y_c$ ) is calculated using eq. 2.3.
- The drop number is also used to calculate the jump-end hydraulic depth ( $y_3$ ):

$$y_3 = d(1.66)Dr^{0.27} \quad (3.3)$$

- This  $y_3$  is used for later comparison with  $T_w$ .

- $y_2$  is then used to calculate drop-point flow conditions:
  - The drop-point Froude number ( $Fr_2$ ) is calculated using eq. 1.12.
  - The drop-point velocity ( $v_2$ ) is then calculated:

$$v_2 = \frac{q}{y_2} \quad (3.4)$$

- $Dr$  is used yet again to calculate the drop length:

$$L_d = d(4.3)Dr^{0.27} \quad (3.5)$$

- $Fr_2$  is then used to calculate the jump length ( $y_2$ ) using a formula particular to this type of structure:

$$L_j = y_2(220) \tanh\left(\frac{Fr_2 - 1}{22}\right) \quad (3.6)$$

- The total basin length starting at downstream face of the weir ( $L_B$ ) is then the sum of the drop and jump lengths:

$$L_B = L_d + L_j \quad (3.7)$$

This process is carried out for several trial values of  $z_2$  until all constraints are met, as discussed in the following section.

### 3.2) Trial Results

Table 3.1 presents this process for the initial  $z_2 = 819.34 \text{ ft}$ . While the hydraulic jump is contained within the system, given  $\frac{T_w - z_2}{y_3} > 1$  for all discharges, the  $Fr_2$  values span the ranges of multiple basin types. This can be determined regarding table 1.7. Therefore, one of the basin types of concern, type IV, is chosen and used for further calculation.

FF	Q	Dr	$y_2$	$y_c$	$y_3$	$\frac{y_3}{T_w - z_2}$	$Fr_2$	$v_2$	$L_d$	$L_j$	$L_B$
5	4,700	0.007	1.026	2.993	6.797	0.909	4.979	28.620	17.607	40.394	58.002
25	7,600	0.018	1.544	4.123	8.811	0.934	4.362	30.760	22.825	51.518	74.343
50	8,900	0.025	1.766	4.580	9.596	0.939	4.177	31.497	24.856	55.716	80.572
100	10,200	0.033	1.983	5.016	10.329	0.942	4.023	32.148	26.755	59.574	86.329
500	13,500	0.058	2.517	6.047	12.017	0.939	3.725	33.528	31.127	68.217	99.345

Table 3.1: Option 3 trial results for  $z_2 =$  natural downstream elevation

It is recommended that for type IV basins, the post-jump head be *at least* 10% below the tailwater depth. That is, for all discharges:

$$\frac{y_3}{T_w - z_2} \leq 0.9 \quad (3.8)$$

Regarding these values shown in table 3.1, an invert elevation equal to natural downstream depth does not meet this condition. Therefore, a deeper  $z_2$  is attempted in order to satisfy this condition.

Table 3.2 presents another trial process for a slightly lower  $z_2 = 818.84 \text{ ft}$ . However, it can be seen that the tailwater safety condition violation persists for the 50, 100 and 500 year events.

FF	Q	Dr	y <sub>2</sub>	y <sub>c</sub>	y <sub>3</sub>	$\frac{y_3}{T_w - z_2}$	Fr <sub>2</sub>	v <sub>2</sub>	L <sub>d</sub>	L <sub>j</sub>	L <sub>B</sub>
5	4,700	0.006	1.018	2.993	6.838	0.857	5.044	28.870	17.713	40.687	58.401
25	7,600	0.017	1.531	4.123	8.864	0.893	4.419	31.028	22.962	51.928	74.890
50	8,900	0.023	1.751	4.580	9.653	0.901	4.232	31.772	25.006	56.173	81.179
100	10,200	0.030	1.966	5.016	10.391	0.906	4.076	32.428	26.916	60.076	86.992
500	13,500	0.053	2.495	6.047	12.089	0.909	3.773	33.820	31.315	68.827	100.140

Table 3.2: Option 3 trial results for  $z_2 = 818.84 \text{ ft}$

Table 3.3 presents the process for which conditions are met, including the type IV 10% safety tailwater offset. This is achieved with an invert elevation of  $z_2 = 818.68 \text{ ft}$ .

FF	Q	Dr	y <sub>2</sub>	y <sub>c</sub>	y <sub>3</sub>	$\frac{y_3}{T_w - z_2}$	Fr <sub>2</sub>	v <sub>2</sub>	L <sub>d</sub>	L <sub>j</sub>	L <sub>B</sub>
5	4,700	0.006	1.015	2.993	6.851	0.842	5.064	28.949	17.747	40.779	58.526
25	7,600	0.016	1.527	4.123	8.881	0.880	4.437	31.112	23.005	52.056	75.061
50	8,900	0.022	1.746	4.580	9.672	0.889	4.249	31.858	25.053	56.316	81.369
100	10,200	0.029	1.961	5.016	10.410	0.895	4.093	32.516	26.967	60.233	87.200
500	13,500	0.052	2.488	6.047	12.112	0.900	3.789	33.913	31.374	69.018	100.390

Table 3.3: Option 3 trial results for  $z_2 = 818.68 \text{ ft}$

The discharge event requiring the largest basin length is used for system sizing as well as flow conditions for feature sizing.

$$L_{B_{opt3}} = 100.390 \text{ ft}$$

### 3.3) Basin Feature Sizing

Figure 3.2 presents the recommended feature sizing for this type IV basin. It should be noted this design will forgo the inclusion of the chute blocks shown. It is assumed that they can be neglected given that their placement would have minimal dissipation effects at the toe of a nearly vertical slope. That is, their location is too far before  $L_d$  to function properly. Table 3.4 presents the dimensions of the remaining feature to be sized - the end sill.

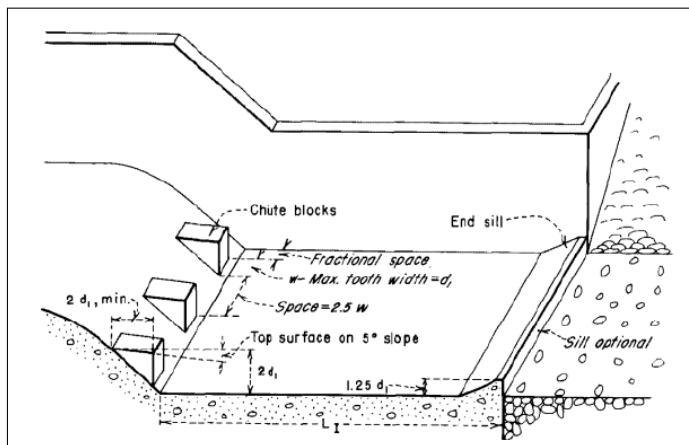


Figure 3.2: Feature detail for type IV basin (USBR)

Description	Symbol	Method of Determination	Value (ft)
end sill height above invert elevation	-	$1.25d_1 = 1.25y_1$	3.11

Table 3.4: Option 3, type IV basin feature sizes

### 3.4) Cost

With the same slab thickness of  $t = 4 \text{ ft}$ , the concrete volume needed for option 3 is calculated as follows:

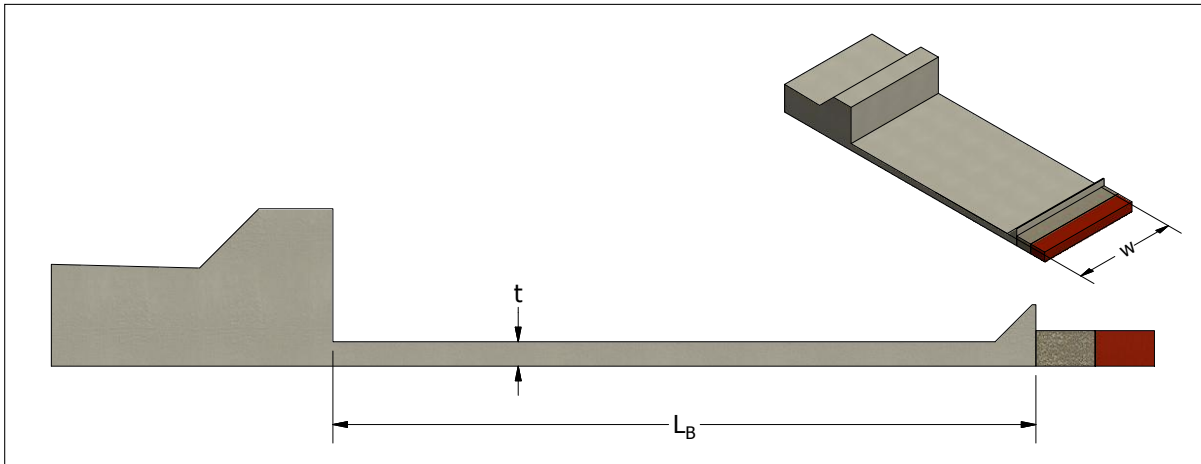


Figure 3.3: Option 3 concrete cost detail

Using the dimensions represented in figure 3.3, the total volume of concrete and excavation is calculated:

$$V_{\text{concrete}} = V_{\text{basin}} = L_B(t)w = \boxed{64,250 \text{ ft}^3}$$
$$V_{\text{excavation}} = [t + (\text{natural downstream } z - z_2)](L_{SB})(w) = \boxed{74,852 \text{ ft}^3}$$

### 3.5) Tailwater - Conjugate Depth Plot

Finally, as shown in figure 3.4, the post-hydraulic-jump depth of  $y_3$  does not cause the water surface to rise above tailwater elevation for drop structure. In fact, there is a sizeable safety distance for all discharges.

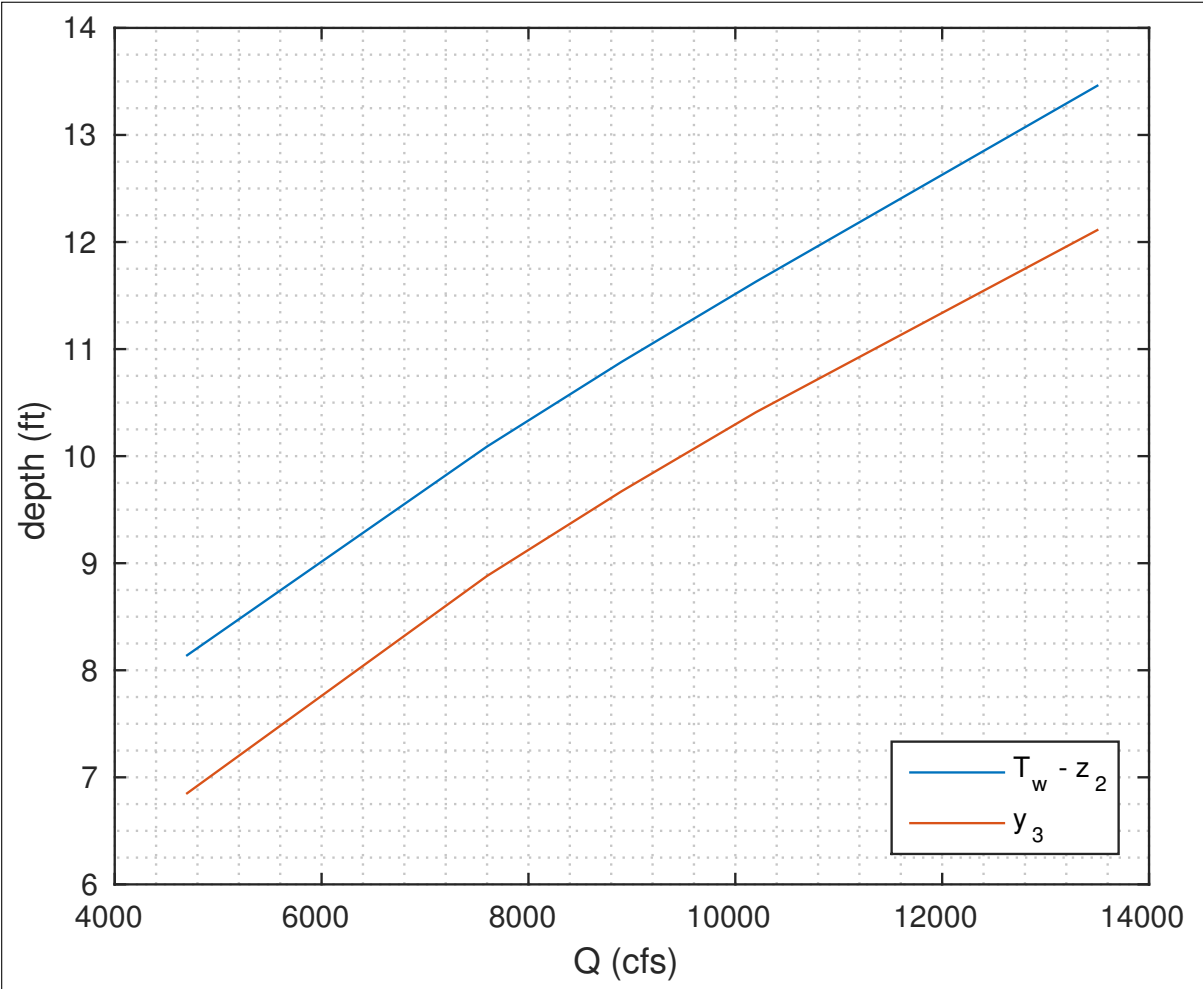


Figure 3.4: Option 3 plot of tailwater and conjugate depths

## 4) Option Choice

Table 4.1 presents the most salient sizes and properties for each option. As discussed below, it can be seen that each option has their own respective economic trade-offs.

Option	Description	$z_2$ (ft)	$L_{SB}$ (ft)	$L_B$ (ft)	$V_{concrete}$ (ft <sup>3</sup> )	$V_{excavation}$ (ft <sup>3</sup> )
1	sloped, basin type III	818.832	29.466	69.711	70,689	21,254
2	stepped, basin type III	817.930	31.063	70.163	76,184	26,888
3	drop, basin type IV	818.680	100.390	100.390	64,250	74,852

Table 4.1: Relevant properties for each option

- Option 1:
  - pro: least total system length;
  - pro: less concrete volume than option 2;
  - pro: least volume excavated;
  - con: greater concrete volume than option 3;
  - con: highest adjustment to invert elevation of all options;
- Option 2:
  - pro: less excavation volume than option 3;
  - pro: occupies less total system length than the drop structure;
  - pro: least adjustment of the invert elevation against natural, downstream elevation;
  - con: more excavation volume than option 1;
  - con: most concrete volume of all options;
  - con: most complicated construction of all options
- Option 3:
  - pro: less adjustment to invert elevation than option 1;
  - pro: least concrete volume of all options;
  - pro: simplest construction of all options;
  - pro: robust containment of hydraulic jump resulting from safety offset between  $y_3$  and  $T_w-2$ ;
  - con: most excavation of all options;
  - con: greatest system length of all options by far;
  - con: flow may occur for which different basin types are recommended

Table 4.2 presents an extremely rough estimate of each option’s construction costs. This table converts each option’s concrete and excavation estimates from table 4.1 into costs based on researched average volumetric pricing.

	<b>Concrete Costs</b>	<b>Excavation Cost</b>	<b>Total</b>
cost per cubic yard	\$ 93	\$ 90	
<b>Option 1</b>	\$243,484	\$70,847	\$314,331
<b>Option 2</b>	\$262,412	\$89,627	\$352,038
<b>Option 3</b>	\$221,306	\$249,507	\$470,812

Table 4.2: Option Pricing

By working with the lightly researched prices per volume for both concrete and excavation, it is clear that option 1 provides the most economic choice. While it does not require the least amount of concrete, it offers dramatic savings on excavation compared to option 1. It’s construction is also quite simple compared to option 2.

Option 1 is chosen

This conclusion was determined with an assumed price per volume. In order to assess option 1’s economic superiority across ranging volumetric prices, several extreme cases are tested. Given that option 3’s biggest pitfall is its high excavation volume, the case of dramatically reduced excavation is assessed in table 4.3. As shown, it is only at an unrealistically low excavation price that option 3 becomes preferable. Thus, the choice of option 1 is determined to be a confident one.

	<b>Concrete Costs</b>	<b>Excavation Cost</b>	<b>Total</b>
cost per cubic yard	\$ 93	<b>\$ 10.8</b>	
Option 1	\$243,484	\$8,502	\$251,986
Option 2	\$262,412	\$10,755	\$273,167
Option 3	\$221,306	\$29,941	\$251,246

Table 4.3: Option Pricing for Extreme Case

It is assumed that when approaching the other extreme, in which the ratio  $\text{cost}_{\text{excavation}}/\text{cost}_{\text{concrete}}$  increases, option 1 only becomes more preferable.

# A Appendix

## A.1 Tailwater - Conjugate Depth Plots for All Options

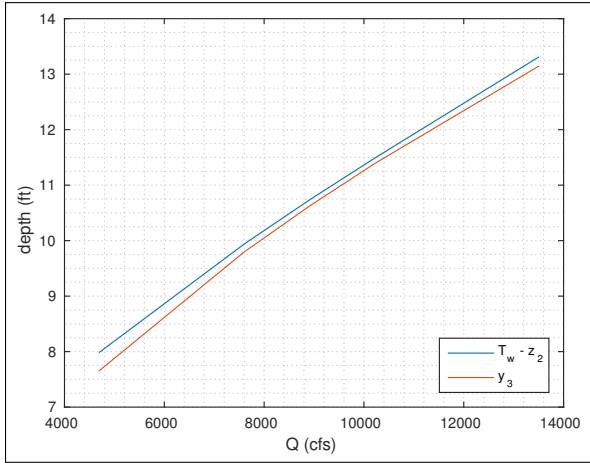


Figure A.1: Option 1

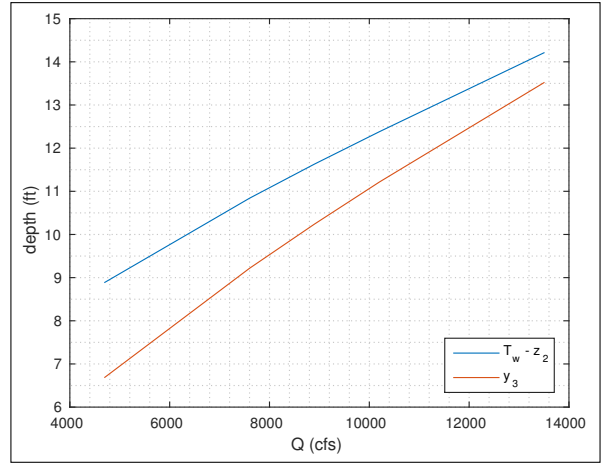


Figure A.2: Option 2

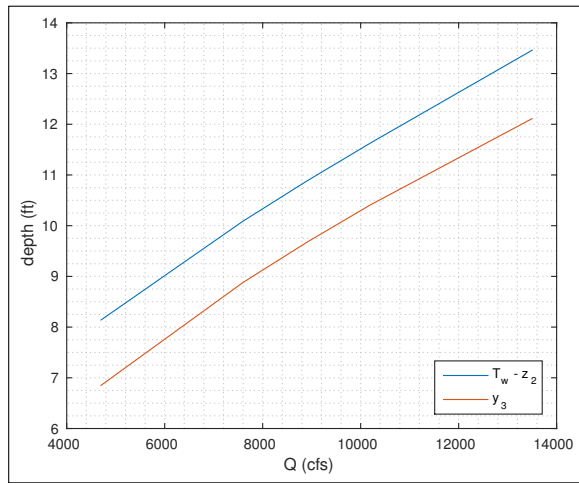


Figure A.3: Option 3



## A.2 Charts and Figures Used

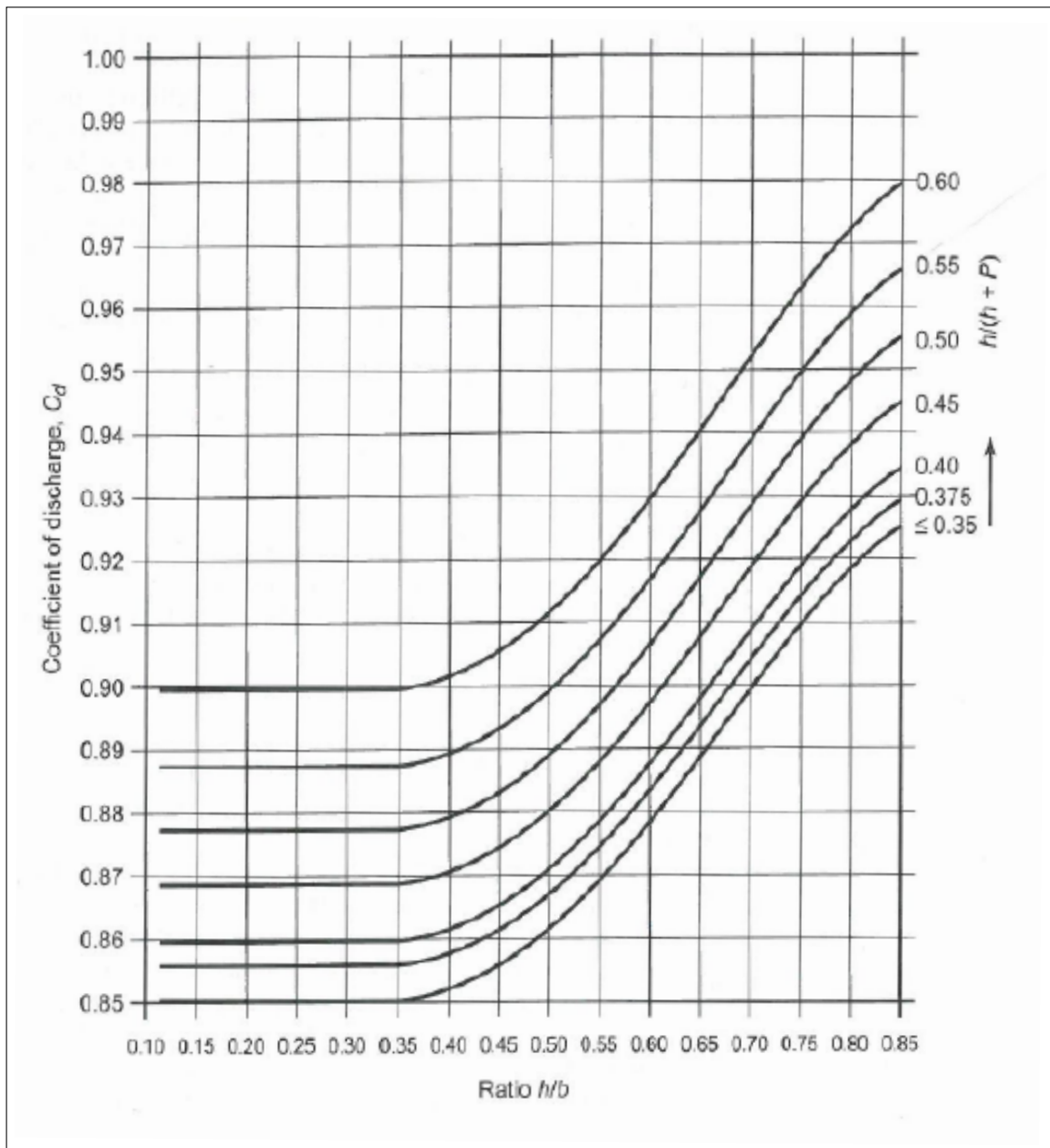


Figure A.4: Discharge Coefficients for broad-crested weirs (British Standards Institution, 1969)

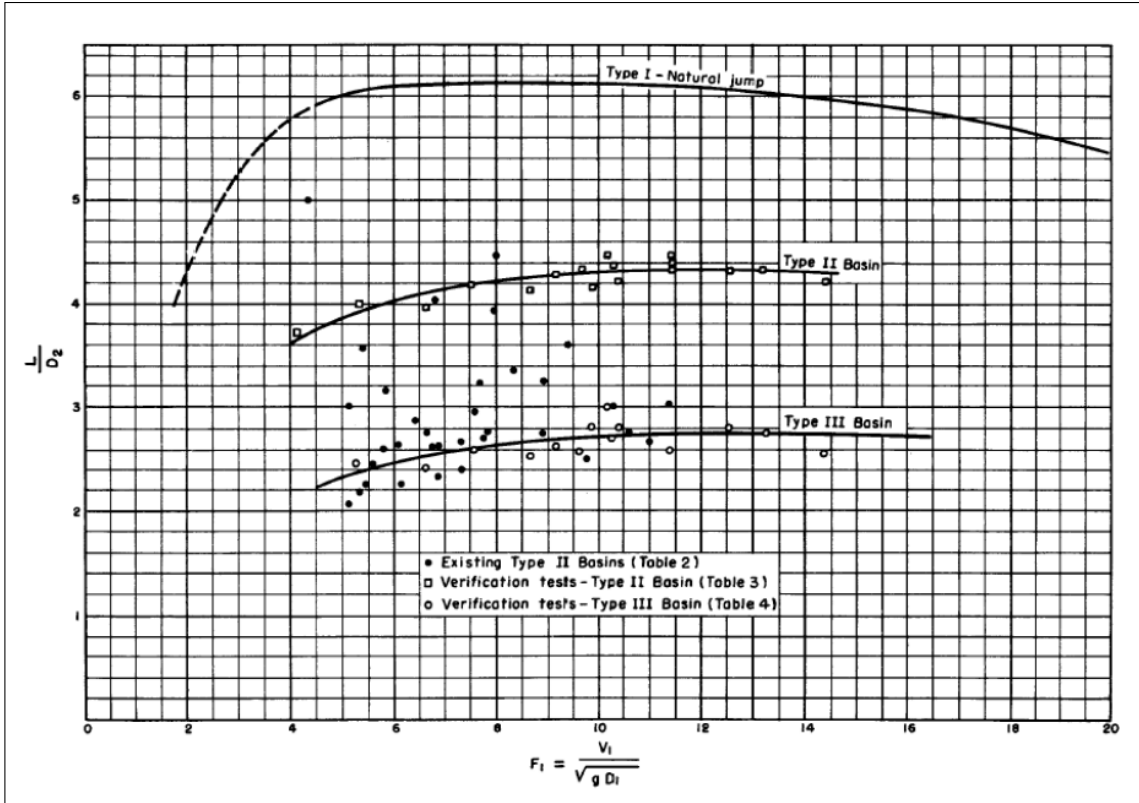


Figure A.5: Stilling basin type III reading (USBR)

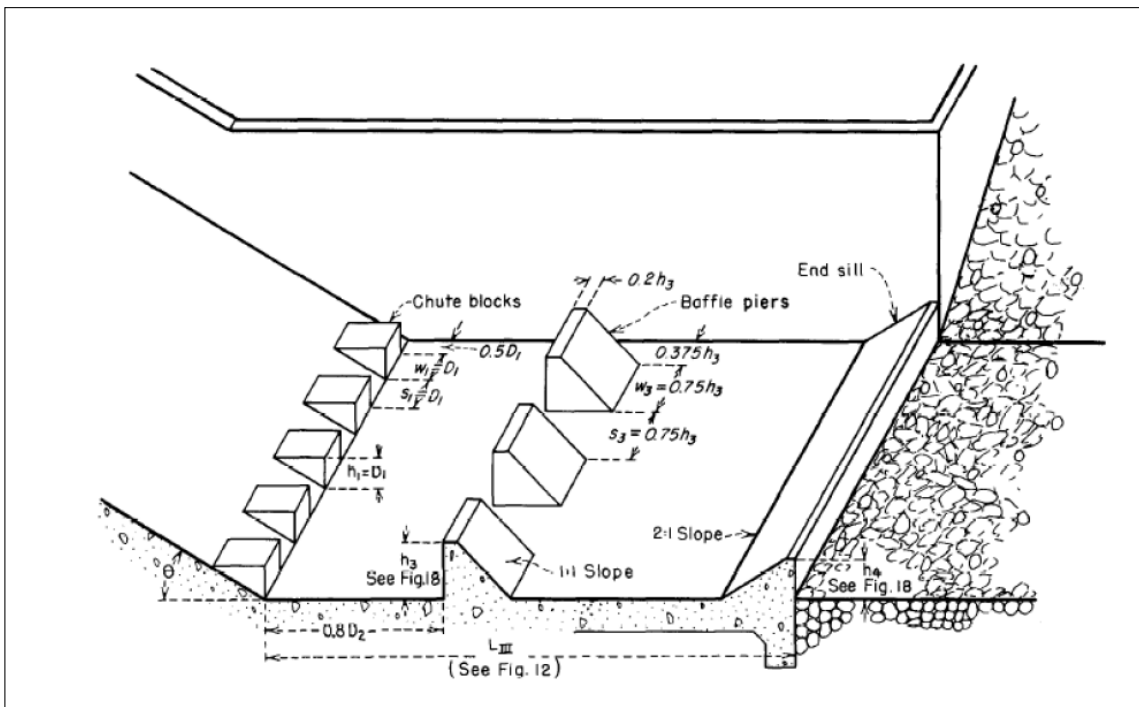


Figure A.6: Feature detail for type III basin (USBR)

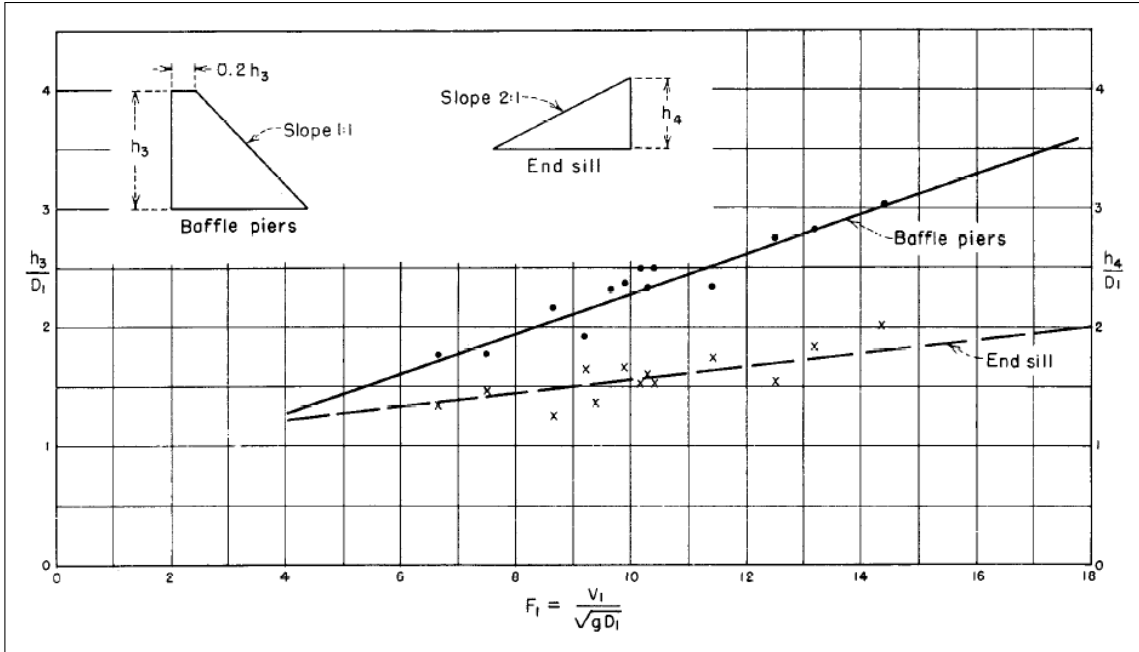


Figure A.7: Chart for  $h_3/D_1$  and  $h_4/D_1$  as a function of  $Fr_2$

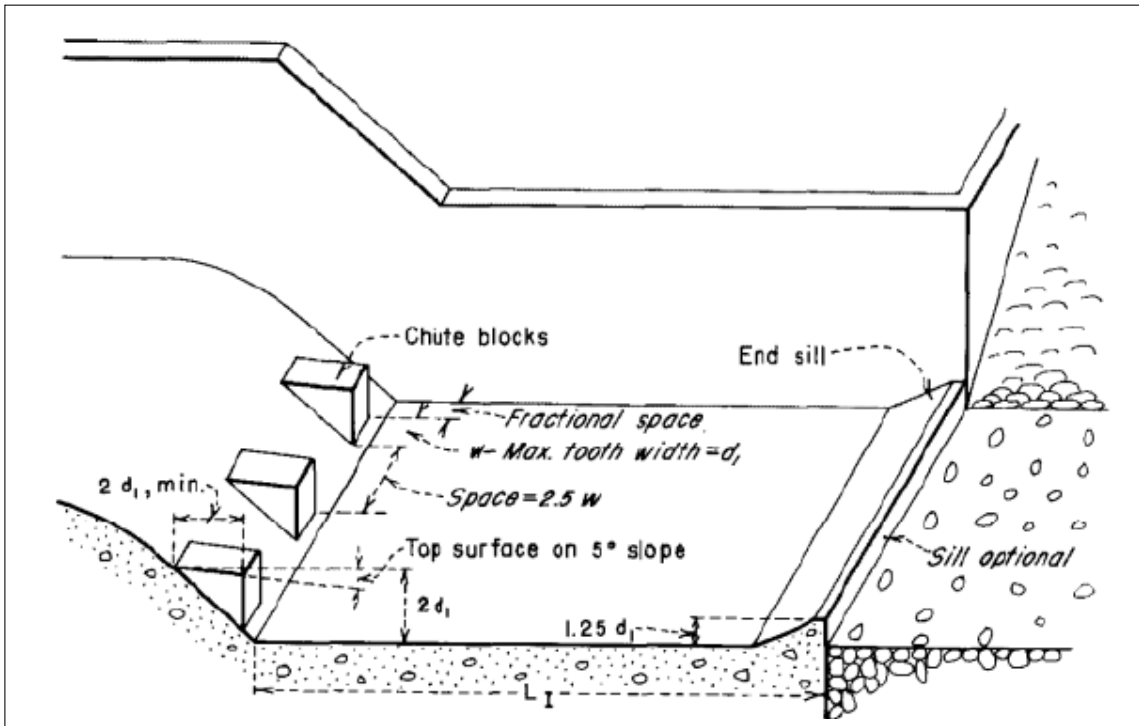


Figure A.8: Feature detail for type IV basin (USBR)

## A.3 Matlab® Scripts

### A.3.1 Option 1 Master Script

```
1 clc
2 clear all
3 clear all
4
5 % --- given properties -----
6 elev_crest = 834.93; % crest elevation [ft]
7 elev_bed_u = 826.93; % upstream bed elevation [ft]
8 elev_bed_d = 819.34;
9 v = 160; % system width [ft]
10 b = 10; % crest breadth [ft]
11
12 % --- given constants -----
13 g = 32.2; % acceleration of gravity [ft/s^2]
14 Ks = 0.002; % concrete roughness [ft]
15
16 % --- preliminary calcs -----
17 P = elev_crest - elev_bed_u; % crest height above upstream bed [ft]
18 alpha = atan(1/2.5); % ds face angle against horizontal (rad)
19
20 % --- crest head elevation calc -----
21
22 % for ff (years) = 5 25 50 100 500
23 Q_array = [4700 7600 8900 10200 13500];
24 Tw_array = [826.82 828.77 829.56 830.31 832.14];
25
26 h_array = zeros(1,length(Q_array));
27 for ii = 1:length(Q_array)
28     [, h_array(ii)] = func_opt1_h_from_Q(Q_array(ii),P,b,w,g, 1);
29 end
30
31 print('101','-depsc2','-r300');
32
33
34 % --- head loss calc -----
35
36 % --- preliminary elevation and array setup -----
37
38 z1 = elev_crest;
39
40 q1_array = zeros(1,length(Q_array));
41 y2_array = zeros(1,length(Tw_array));
42 %z2_array = zeros(1,length(Q_array));
43 h1_headloss_array = zeros(1,length(Q_array));
44 h1_energy_array = zeros(1,length(Q_array));
45 v2_array = zeros(1,length(Q_array));
46 L_loss_array = zeros(1,length(Q_array));
47 del1_array = zeros(1,length(Q_array));
48 del3_array = zeros(1,length(Q_array));
49 Fr2_array = zeros(1,length(Q_array));
50 y3_array = zeros(1,length(Q_array));
51 y3_plus_z2_array = zeros(1,length(Q_array));
52 L_over_y2_array = zeros(1,length(Q_array));
53 L_array = zeros(1,length(Q_array));
54 Dr_array = zeros(1,length(Q_array));
55 Ld_array = zeros(1,length(Q_array));
56
57 for ii = 1:length(Q_array)
58     q1_array(ii) = Q_array(ii)/w;
59 end
60
61 % --- determine z2 compatible with all flows -----
62
63 tol = 0.1; % threshold distance by which Tw exceeds y3 + z2 [ft]
64
65 z2=func_z2_solver(b,z1,alpha,q1_array,h_array,Ks,g,...
66     Tw_array,elev_bed_d,0,tol);
67
68
69 % --- y2 solve from Tw - store results -----
70
71
72 L_loss = b + (z1 - z2)/sin(alpha);
73 del1 = L_loss*0.02*(L_loss/Ks)^(-0.233);
74 del3 = 0.22*del1;
75
76 for ii = 1:length(Q_array)
77     y2_array(ii)=...
78         func_y2_solver_with_known_z2(z1,z2,q1_array(ii),h_array(ii),g,del3);
79
80     h1_headloss_array(ii)= h_array(ii) + z1- y2_array(ii) - ...
81         ((q1_array(ii)/y2_array(ii))^2)/(2*g) - z2;
82     L_loss_array(ii) = b + (z1 - z2)/sin(alpha);
83     del1_array(ii) = L_loss_array(ii)*0.02*(L_loss_array(ii)/Ks)^(-0.233);
84     del3_array(ii) = 0.22*del1_array(ii);
85     v2_array(ii) = q1_array(ii)/y2_array(ii);
86     h1_energy_array(ii)= del3_array(ii)*(v2_array(ii)^3)/(2*g*q1_array(ii));
87
88     disp({'h1_headloss',num2str(h1_headloss_array(ii))});
89     disp({'h1_energy_array',num2str(h1_energy_array(ii))});
90
91     assert(abs(h1_headloss_array(ii) - h1_energy_array(ii)) < 0.001,'h1 check');
92
93     Fr2_array(ii) = (q1_array(ii)/y2_array(ii))/sqrt(g*y2_array(ii));
94     y3_array(ii) = y2_array(ii)*0.5*(sqrt(1 + 8*Fr2_array(ii)^2) - 1);
95     y3_plus_z2_array(ii) = y3_array(ii) + z2;
96     disp(y3_plus_z2_array(ii));
97     disp(Tw_array(ii));
98
99     assert(Tw_array(ii) - y3_plus_z2_array(ii) > 0,'Tw check')
100
101 % --- calc basin length -----
102 L_over_y2_array(ii) = func_opt1_L_over_y2_from_Fr2(Fr2_array(ii), 0);
103 L_array(ii) = L_over_y2_array(ii)*y3_array(ii);
104 end
105
106 % --- output results -----
107 opt1_table1 = zeros(length(Q_array),13);
108 opt1_table1(:,1) = Q_array;
109 opt1_table1(:,2) = q1_array;
110 opt1_table1(:,3) = y2_array;
111 opt1_table1(:,4) = z2;
112 opt1_table1(:,5) = h1_headloss_array;
113 opt1_table1(:,6) = v2_array;
114 opt1_table1(:,7) = h1_energy_array;
115 opt1_table1(:,8) = Fr2_array;
116 opt1_table1(:,9) = y3_array;
117 opt1_table1(:,10) = y3_plus_z2_array;
118 opt1_table1(:,11) = Tw_array;
119 opt1_table1(:,12) = L_over_y2_array;
120 opt1_table1(:,13) = L_array;
121
122 xlswrite('opt1_table',opt1_table1);
123
124 % --- check Ld -----
125
126 for ii = 1:length(Dr_array);
127     Dr_array(ii) = q1_array(ii)^2/(g*(z1 - z2)^3);
128     Ld_array(ii) = 4.3*(z1-z2)*Dr_array(ii)^0.27;
129 end
130
131 Ld_table = zeros(length(Q_array),4);
132 Ld_table(:,1) = [5 25 50 100 500];
133 Ld_table(:,2) = Q_array;
134 Ld_table(:,3) = Dr_array;
135 Ld_table(:,4) = Ld_array;
136 %xlswrite('Ld_table',Ld_table);
137
138 % --- sizing basin features -----
139 opt1_sizing_table = zeros(11, 1);
140
141 D1 = y2_array(5);
142 D2 = y3_array(5);
143 Fr2_array = [ 6.9085 5.7099 5.3652 5.0861 4.5767];
144
145 opt1_sizing_table(1,1) = D1; %h1
146 opt1_sizing_table(2,1) = D1; %s1
147 opt1_sizing_table(3,1) = D1; %w1
148 opt1_sizing_table(4,1) = 0.5*D1; % cb_wall_gap
149 opt1_sizing_table(5,1) = 0.8*D2; % toe_bp_spacing
150 h3 = func_h3_over_D1_from_Fr2(Fr2_array(5),0)*D1;
151 h4 = func_h4_over_D1_from_Fr2(Fr2_array(5),0)*D1;
152 opt1_sizing_table(6,1) = h3; %h3
153 opt1_sizing_table(7,1) = h4; %h4
154 opt1_sizing_table(8,1) = 0.375*h3; %bp_wall_gap
155 opt1_sizing_table(9,1) = 0.2*h3; %bp_top_width
156 opt1_sizing_table(10,1) = 0.75*h3; %w3
157 opt1_sizing_table(11,1) = 0.75*h3; %s3
158
159 c1c
160 %xlswrite('opt1_sizing_table',opt1_sizing_table);
161
162 % --- basin cost -----
163
164 t = 4;
165 conc_slope_volume = 0.5*(z1 - z2)*(z1-z2)*2.5*w;
166 con_basin_volume = t*L_array(5)*w;
167 opt1_total_conc_volume = conc_slope_volume + con_basin_volume;
168 exc_volume = (elev_bed_d - z2 + t)*L_array(5)*w;
169
170 % --- plot y3 and Tw vs Q -----
171
172 figure; axis equal
173 plot(Q_array, Tw_array - z2); hold on
174 plot(Q_array, y3_array);
175 legend('Tw - z2','y3','location','southeast');
176 xlabel('Q (cfs)'); ylabel('depth (ft)');
177 grid minor;
178
179 print('105','-depsc2','-r300');
```

## A.3.2 Option 2 Master Script

```

1  clc
2  clear all
3  clear all
4
5  % --- given properties -----
6  elev_crest = 834.93; % crest elevation [ft]
7  z1 = elev_crest;
8  elev_bed_u = 826.93; % upstream bed elevation [ft]
9  elev_bed_d = 819.34;
10 w = 160; % system width [ft]
11 b = 10; % crest breadth [ft]
12
13 % --- given constants -----
14 g = 32.2; % acceleration of gravity [ft/s^2]
15 Ks = 0.002; % concrete roughness [ft]
16 k = 4.5; % for stepped spillway equations
17
18 % --- preliminary calcs -----
19 P = elev_crest - elev_bed_u; % crest height above upstream bed [ft]
20
21 % --- crest head elevation calc -----
22
23 % for ff (years) = 5 25 50 100 500
24 Q_array = [4700 7600 8900 10200 13500];
25 Tw_array = [826.82 828.77 829.56 830.31 832.14];
26 ql_array = Q_array/w;
27 yc_array = zeros(1,length(Q_array));
28 yc_over_h_array = zeros(1,length(Q_array));
29 y2_array = zeros(1,length(Q_array));
30 v2_array = zeros(1,length(Q_array));
31 Fr2_array = zeros(1,length(Q_array));
32 y3_array = zeros(1,length(Q_array));
33 Tw_depth_array = zeros(1,length(Q_array));
34 Tw_over_y3_array = zeros(1,length(Q_array));
35 L_over_y2_array = zeros(1,length(Q_array));
36 LB_array = zeros(1,length(Q_array));
37 Dr_array = zeros(1,length(Q_array));
38 LD_array = zeros(1,length(Q_array));
39 LD_over_Ls_array = zeros(1,length(Q_array));
40
41 % --- determination of parameters -----
42 h = 1;
43 s = 2.3;
44 N = 17;
45
46 z2 = z1 - N*h;
47 alpha = atan(h/s); %radians
48 cmean = 1.44*sin(alpha) - 0.08;
49 fd = 2/sqrt(pi)/k;
50 fc = fd*0.5*(1 + tanh(2.5*(0.5 - cmean)/(cmean*(1-cmean)))));
51 yc_over_h_nappe = 0.89 - 0.4*h/s;
52 yc_over_h_skim = 1.2 - 0.325*h/s;
53
54 for ii = 1:length(Q_array)
55 yc_array(ii) = (ql_array(ii)^2/g)^(1/3);
56 yc_over_h_array(ii) = yc_array(ii)/h;
57 assert(yc_over_h_array(ii) > yc_over_h_skim, 'not skimming flow');
58 y2_array(ii) = yc_array(ii)*(fc/(8*sin(alpha)))^(1/3);
59 v2_array(ii) = ql_array(ii)/y2_array(ii);
60 if v2_array(ii) > 60
61 disp('not type III - v2');
62 end
63 Fr2_array(ii) = v2_array(ii)/sqrt(y2_array(ii)*g);
64 if Fr2_array(ii) < 4.5
65 disp('not type III - Fr');
66 end
67 y3_array(ii) = y2_array(ii)*0.5*(sqrt(1+ 8*Fr2_array(ii)^2) - 1);
68 Tw_depth_array(ii) = Tw_array(ii) - z2;
69 Tw_over_y3_array(ii) = Tw_depth_array(ii)/(y3_array(ii));
70 if Tw_over_y3_array(ii) < 1
71 disp('HJ uncontained');
72 end
73
74 L_over_y2_array(ii) = func_opt1_L_over_y2_from_Fr2(Fr2_array(ii), 0);
75 LB_array(ii) = L_over_y2_array(ii)*y3_array(ii);
76 Dr_array(ii) = ql_array(ii)^2/(g*(z1 - z2)^3);
77 LD_array(ii) = 4.3*(z1 - z2)*Dr_array(ii)^(0.27);
78 LD_over_Ls_array(ii) = LD_array(ii)/(N*s);
79 if LD_over_Ls_array(ii) > 1
80 disp('Ld exceeds');
81 end
82 end
83
84 opt2_table = zeros(length(Q_array), 18);
85 opt2_table(:,1) = [ 5 25 50 100 500 ];
86 opt2_table(:,2) = Q_array;
87 opt2_table(:,3) = ql_array;
88 opt2_table(:,4) = yc_array;
89 opt2_table(:,5) = yc_over_h_array;
90 opt2_table(:,6) = yc_over_h_nappe*ones(1,length(Q_array));
91 opt2_table(:,7) = yc_over_h_skim*ones(1,length(Q_array));
92 opt2_table(:,8) = y2_array;
93 opt2_table(:,9) = v2_array;
94 opt2_table(:,10) = Fr2_array;
95 opt2_table(:,11) = y3_array;
96 opt2_table(:,12) = Tw_depth_array;
97 opt2_table(:,13) = Tw_over_y3_array;
98 opt2_table(:,14) = L_over_y2_array;
99 opt2_table(:,15) = LB_array;
100 opt2_table(:,16) = Dr_array;
101 opt2_table(:,17) = LD_array;
102 opt2_table(:,18) = LD_over_Ls_array;
103 %xlswrite('opt2_table03',opt2_table)
104
105 % --- sizing basin features -----
106 opt2_sizing_table = zeros(11, 1);
107
108 D1 = y2_array(5);
109 D2 = y3_array(5);
110
111 opt2_sizing_table(1,1) = D1; %h1
112 opt2_sizing_table(2,1) = D1; %s1
113 opt2_sizing_table(3,1) = D1; %w1
114 opt2_sizing_table(4,1) = 0.5*D1; % cb_wall_gap
115 opt2_sizing_table(5,1) = 0.8*D2; % toe_bp_spacing
116 h3 = func_h3_over_D1_from_Fr2(Fr2_array(5),0)*D1;
117 h4 = func_h4_over_D1_from_Fr2(Fr2_array(5),0)*D1;
118 opt2_sizing_table(6,1) = h3; %h3
119 opt2_sizing_table(7,1) = h4; %h4
120 opt2_sizing_table(8,1) = 0.375*h3; %bp_wall_gap
121 opt2_sizing_table(9,1) = 0.2*h3; %bp_top_width
122 opt2_sizing_table(10,1) = 0.75*h3; %w3
123 opt2_sizing_table(11,1) = 0.75*h3; %s3
124
125 clc
126 xlswrite('opt2_sizing_table',opt2_sizing_table);
127
128 % --- basin cost -----
129 t = 4;
130 conc_slope_volume = 0.5*(s*N)*(h*N)*w;
131 conc_steps_volume = 0.5*(s*h)*N*s;
132 conc_basin_volume = t*(LB_array(5))*w;
133 cxc_basin_volume = (t + (elev_bed_d - z2))*LB_array(5)*w;
134
135 % --- plot y3 and Tw vs Q -----
136
137 figure; axis equal
138 plot(Q_array, Tw_array - z2); hold on
139 plot(Q_array, y3_array);
140
141 legend('T_w - z_2','y_3','location','southeast');
142 xlabel('Q (cfs)'); ylabel('depth (ft)');
143 grid minor;
144
145 print('106','-depsc2','-r300');

```

### A.3.3 Option 3 Master Script

```

1  clc
2  clear all
3  clear all
4
5  % --- given properties -----
6  elev_crest = 834.93;           % crest elevation [ft]
7  z1 = elev_crest;
8  elev_bed_u = 826.93;         % upstream bed elevation [ft]
9  elev_bed_d = 819.34;
10 w = 160;                     % system width [ft]
11 b = 10;                      % crest breadth [ft]
12
13 % --- given constants -----
14 g = 32.2;                   % acceleration of gravity [ft/s^2]
15 Ks = 0.002;                 % concrete roughness [ft]
16 k = 4.5;                    % for stepped spillway equations
17
18 % --- preliminary calcs -----
19 P = elev_crest - elev_bed_u; % crest height above upstream bed [ft]
20
21 % --- crest head elevation calc -----
22
23 z2 = elev_bed_d - 0.66;
24 d = z1 - z2;
25
26 % for ff (years) =          5      25      50      100      500
27 Q_array = [4700 7600 8900 10200 13500];
28 Tw_array = [826.82 828.77 829.56 830.31 832.14];
29 ql_array = Q_array/w;
30 Dr_array = zeros(1,length(Q_array));
31 y2_array = zeros(1,length(Q_array));
32 yc_array = zeros(1,length(Q_array));
33 y3_array = zeros(1,length(Q_array));
34 y3_over_Tw_array = zeros(1,length(Q_array));
35 Fr2_array = zeros(1,length(Q_array));
36 V2_array = zeros(1,length(Q_array));
37 LD_array = zeros(1,length(Q_array));
38 Lj_array = zeros(1,length(Q_array));
39 LB_array = zeros(1,length(Q_array));
40
41 for ii = 1:length(Q_array)
42   Dr_array(ii) = (ql_array(ii)^2)/(g*(z1 - z2)^3);
43   y2_array(ii) = d*0.54*Dr_array(ii)^0.425;
44   yc_array(ii) = (ql_array(ii)^2/g)^(1/3);
45   y3_array(ii) = d*1.66*Dr_array(ii)^0.27;
46   y3_over_Tw_array(ii) = y3_array(ii)/(Tw_array(ii) - z2);
47   disp(y3_over_Tw_array(ii));
48   if y3_over_Tw_array(ii) > 1
49     disp('HJ not contained');
50   end
51
52   V2_array(ii) = ql_array(ii)/y2_array(ii);
53   LD_array(ii) = d*4.3*Dr_array(ii)^0.27;
54   Fr2_array(ii) = ql_array(ii)/sqrt(g*y2_array(ii)^3);
55   Lj_array(ii) = y2_array(ii)*220*tanh(((Fr2_array(ii) - 1)/22));
56   LB_array(ii) = Lj_array(ii) + LD_array(ii);
57 end
58
59 opt3_table = zeros(length(Q_array),12);
60 opt3_table(:,1) = [ 5      25      50      100      500      ];
61 opt3_table(:,2) = Q_array;
62 opt3_table(:,3) = Dr_array;
63 opt3_table(:,4) = y2_array;
64 opt3_table(:,5) = yc_array;
65 opt3_table(:,6) = y3_array;
66 opt3_table(:,7) = y3_over_Tw_array;
67 opt3_table(:,8) = Fr2_array;
68 opt3_table(:,9) = V2_array;
69 opt3_table(:,10) = LD_array;
70 opt3_table(:,11) = Lj_array;
71 opt3_table(:,12) = LB_array;
72
73 xlswrite('opt3_table03',opt3_table);
74
75 % --- feature sizing -----
76
77 endsill = 1.25*y2_array(5);
78
79 % --- basin cost -----
80
81 t = 4;
82 conc_basin_volume = t*(LB_array(5))*w;
83 cxc_basin_volume = (t + (elev_bed_d - z2))*LB_array(5)*w;
84
85 % --- plot y3 and Tw vs Q -----
86
87 figure; axis equal
88 plot(Q_array, Tw_array - z2); hold on
89 plot(Q_array, y3_array);
90
91 legend('T_w - z_2','y_3','location','southeast');
92 xlabel('Q (cfs)'); ylabel('depth (ft)');
93 grid minor;
94
95 print('-r300');

```

### A.3.4 Function: $h = f(Q)$ Solver

```

1 function [Cd_out, h_out] = func_opt1_h_from_Q(Q,P,b,w,g, plot_fig)
2
3 % --- set axis scaling -----
4
5 ori = [68 547];
6 x_end = 449;
7 y_top = 69;
8
9 pivot_x = 199;
10
11 scale_xx = (x_end - ori(1))/(0.85 - 0.10);
12 scale_yy = (ori(2) - y_top)/(1 - 0.85);
13
14 % --- record points for interpolation -----
15
16 int_mat_xx = [pivot_x; 247; 323; 425; 450];
17
18 int_mat_yy = ...
19 [546 528 516 487 460 428 388;
20 528 510 498 469 441 409 370;
21 455 439 426 395 365 332 293;
22 328 314 298 265 232 198 156;
23 306 293 277 243 210 176 133;
24 .350 .375 .400 .450 .500 .550 .600];
25
26
27 % --- create matrix of interpolated values -----
28
29 [rows, cols] = size(int_mat_yy);
30 AA = ones(rows-1, rows-1);
31
32 for ii = 1:rows-1
33     for jj = 1:rows-1
34         AA(ii, jj) = int_mat_xx(ii)^(rows-1-jj);
35     end
36 end
37
38 CC = zeros(rows-1, cols);
39
40 for kk = 1:cols
41     CC(:, kk) = AA \ int_mat_yy(1:rows-1, kk);
42 end
43
44 % -- create matrix of Cd values, from which the output Cd is interpolated -
45
46 N_plot = 1000;
47 He_over_b_array = linspace(ori(1), x_end, N_plot);
48 %He_over_b_array = linspace(ori(1), 532, N_plot);
49 Cd_plot = zeros(cols, N_plot);
50
51 for kk = 1:cols
52     for ii = 1:N_plot
53         if He_over_b_array(ii) < pivot_x
54             Cd_plot(kk, ii) = int_mat_yy(1, kk);
55         else
56             for jj = 1:rows-1
57                 Cd_plot(kk, ii) = Cd_plot(kk, jj) + ...
58                     CC(jj, kk) * He_over_b_array(ii)^(rows-1-jj);
59             end
60         end
61     end
62 end
63
64 % --- interpolate initial output value, with Cd = 0.85 -----
65
66
67 trial_Cd = 1.11;
68 Cd_error = 1.0;
69 tolerance = 1e-4;
70 while Cd_error > tolerance
71     trial_h = (3*Q^(2/3))/(2*(trial_Cd*w)^(2/3)*g^(1/3));
72     trial_h_over_h_plus_P = trial_h/(trial_h + P);
73     trial_h_over_b = trial_h/b;
74     if trial_h_over_h_plus_P < 0.35
75         trial_h_over_h_plus_P = 0.350001;
76     end
77
78     trial_h_over_b_plot = (trial_h_over_b - 0.1) * scale_xx + ori(1);
79
80     for nn = 1:N_plot-1
81         if He_over_b_array(nn) > trial_h_over_b_plot
82             xx_index = nn;
83             break
84         end
85     end
86
87     kk_index = 1;
88     for kk = 1:cols-1
89         if trial_h_over_h_plus_P > int_mat_yy(rows, kk) && ...
90             trial_h_over_h_plus_P < int_mat_yy(rows, kk+1)
91             kk_index = kk;
92             break
93         end
94     end
95
96     trial_Cd_out_plot_L = Cd_plot(kk_index, xx_index);
97     trial_Cd_out_plot_U = Cd_plot(kk_index+1, xx_index);
98
99     trial_Cd_out_plot = (trial_Cd_out_plot_U - trial_Cd_out_plot_L) / ...
100         (int_mat_yy(rows, kk+1) - int_mat_yy(rows, kk)) * ...
101         (trial_h_over_h_plus_P - int_mat_yy(rows, kk)) + ...
102         trial_Cd_out_plot_L;
103
104     trial_Cd_out = 1.0 - (trial_Cd_out_plot - y_top) / scale_yy;
105
106     Cd_error = abs(trial_Cd_out - trial_Cd);
107     trial_Cd = trial_Cd - tolerance/10;
108 end
109
110 Cd_out = trial_Cd;
111 h_out = trial_h;
112
113
114 % --- plot results if requested -----
115 if plot_fig == 1
116
117     I = imread('10.10_read.png');
118     B = imrotate(I, -0.45);
119
120     figure
121     imshow(B); hold on;
122     plot(ori(1), ori(2), 'or');
123     plot([ori(1) x_end], [ori(2) ori(2)], '-k', 'linewidth', 2);
124     plot([ori(1) ori(1)], [y_top ori(2)], '-k', 'linewidth', 2);
125     for kk = 1:cols
126         plot(He_over_b_array, Cd_plot(kk, :), 'r', 'linewidth', 2);
127     end
128
129     for ii = 1:rows-1
130         for jj = 1:length(int_mat_yy)
131             plot(int_mat_xx(ii), int_mat_yy(ii, jj), 'ro');
132             plot(ori(1), int_mat_yy(1, jj), 'ro');
133         end
134     end
135
136     %plot(He_over_b_array(xx_index), trial_Cd_out_plot_L, 'be');
137     %plot(He_over_b_array(xx_index), trial_Cd_out_plot_U, 'be');
138     %plot(He_over_b_array(xx_index), trial_Cd_out_plot, 'co');
139
140     % h1 = plot(He_over_P_array_plot, CT_mat_plot(row_aa, :), '-r', 'linewidth', 2);
141     % h2 = plot(He_over_P_array_plot, CT_mat_plot(row_bb, :), '-b', 'linewidth', 2);
142     % h3 = plot(He_over_P_array_plot, CT_array_out_plot, ':c', 'linewidth', 2);
143     % legend([h1 h2 h3], {'lower \alpha', 'upper \alpha', 'interpolated \alpha'}, ...
144         'location', 'northeast');
145 end
146 end

```

### A.3.5 Function: option 1 $z_2$ Solver

```

1 function[z2_out] = func_z2_solver( b, z1, alpha,ql_array,h_array,...
2                               Ks,g,Tw_array,elev_bed_d,gen_table,tol)
3
4 out_table = zeros(30,7);
5 z2_array = zeros(1,length(ql_array));
6
7 for jj = 1:length(ql_array)
8
9 disp('---');
10 disp({'head-',num2str(h_array(jj))});
11
12     diff = 2;
13     ql = ql_array(jj);
14     h = h_array(jj);
15     Tw = Tw_array(jj);
16
17 z2 = elev_bed_d - 10;
18
19 tt = 1;
20 while diff > tol;
21
22 syms y2
23 % --- calculate head loss according to hl equation -----
24
25 L_loss = b + (z1 - z2)/sin(alpha);
26 del1 = L_loss*0.02*(L_loss/Ks)^(-0.233);
27 del3 = 0.22*del1;
28
29 d = y2;
30 U = ql/d;
31
32 % --- calculate head loss according to energy balance -----
33
34 v2 = ql/y2;
35 hl_headloss = (del3*U^3)/(2*g*ql);
36 hl_energy = h + z1 - y2 - (v2^2)/(2*g) - z2;
37
38 % --- account for hydraulic jump -----
39
40 y2 = double(vpasolve(hl_energy==hl_headloss,y2));
41
42 for ii = 1:length(y2)
43     y2(ii) = (real(sqrt(y2(ii))))^2;
44     if y2(ii) > h
45         y2(ii) = 0;
46     end
47 end
48
49 y2 = sum(y2);
50
51 Fr2 = (ql/y2)/sqrt(g*y2);
52 y3 = y2*0.5*(sqrt(1 + 8*Fr2^2) - 1);
53
54 diff = Tw - (y3 + z2);
55 disp(diff);
56
57 out_table(tt,1) = tt - 1;
58 out_table(tt,2) = z2;
59 out_table(tt,3) = h + z1 - y2 - ((ql/y2)^2)/(2*g) - z2;
60 out_table(tt,4) = y2;
61 out_table(tt,5) = y3;
62 out_table(tt,6) = Tw - (y3+ z2);
63
64 z2 = z2 + diff*0.5;
65
66 out_table(tt,7) = z2;
67 tt = tt + 1;
68
69 end
70
71 z2 = z2 - diff*0.5;
72
73 out_table(tt-1,7) = z2;
74
75 if gen_table == 1 && jj == length(ql_array);
76 xlsxwrite('tab02',out_table);
77 end
78
79 z2_array(jj) = z2;
80
81 end
82 z2_out = min(z2_array);
83
84 end

```

### A.3.6 Function: $y_2 = f(z_2)$ Solver

```

1 function[y2_out]=func_y2_solver_with_known_z2(z1,z2,ql,h,g,del3)
2
3 % --- calculate head loss according to hl equation -----
4
5
6 syms y2
7 U = ql/y2;
8
9 % --- calculate head loss according to energy balance -----
10
11 v2 = ql/y2;
12 hl_headloss = (del3*U^3)/(2*g*ql);
13 hl_energy = h + z1 - y2 - (v2^2)/(2*g) - z2;
14
15 % --- solve for y2 and output -----
16

```

```

17 y2 = double(vpasolve(hl_energy==hl_headloss,y2));
18
19 % --- remove negative solutions for y2 that are negative and above y1 -----
20
21 for ii = 1:length(y2);
22     if y2(ii) > h
23         y2(ii) = 0;
24     end
25     if y2(ii) < 0
26         y2(ii) = 0;
27     end
28 end
29
30 % --- output value -----
31
32 y2_out = sum(y2);
33
34 end

```

### A.3.7 Function: $\frac{L_j}{y_2} = f(\text{Fr}_2)$ Reader

```

1 function [L_over_y2_out] = func_opt1_L_over_y2_from_Fr2(Fr2, plot_fig)
2
3 %plot_fig = 1;
4 %Fr2 = Fr2_array(1);
5
6 % --- set axis scaling -----
7
8 ori = [59 578];
9 x_end = 896;
10 y_top = 68;
11
12 scale_xx = (x_end - ori(1))/20;
13 scale_yy = (ori(2) - y_top)/6;
14
15 % --- record points for interpolation -----
16
17 int = [249 388; 322 367; 436 351; 586 345 ;747 348 ];
18
19 % --- create matrix of interpolated values -----
20
21 [rows, ~] = size(int);
22 AA = ones(rows,rows);
23
24 for ii = 1:rows
25     for jj = 1:rows
26         AA(ii,jj) = int(ii,1)^(rows-jj);
27     end
28 end
29
30 CC = AA\int(:,2);
31
32 % -- create matrix of Cd values, from which the output Cd is interpolated -
33
34 N_plot = 1000;
35
36 Fr1_array = linspace(int(1,1),max(int(:,1)),N_plot);
37 L_over_y1_array = zeros(N_plot);
38
39 for ii = 1:N_plot
40     for jj = 1:rows
41         L_over_y1_array(ii) = L_over_y1_array(ii) + ...
42             CC(jj)*Fr1_array(ii)^(rows-jj);
43     end
44 end
45
46 % --- interpolate initial output value, with Cd = 0.85 -----
47
48 Fr1_unscaled = Fr2;
49 Fr1 = Fr1_unscaled*scale_xx + ori(1);
50
51 L_over_y1 = 0;
52
53 for jj = 1:rows
54     L_over_y1 = L_over_y1 + CC(jj)*Fr1^(rows-jj);
55 end
56
57 L_over_y1_unscaled = 6 - (L_over_y1 - y_top)/scale_yy;
58 L_over_y2_out = L_over_y1_unscaled;
59
60 % --- plot results if requested -----
61 if plot_fig == 1
62
63     figure;
64     I = imread('fig6.png');
65     B = imrotate(I,0.00);
66
67 imshow(B); hold on;
68 plot(ori(1),ori(2),'or');
69 plot([ori(1) x_end],[ori(2) ori(2)],'-b','linewidth',2);
70 plot([ori(1) ori(1)],[y_top ori(2)],'-r','linewidth',2);
71
72 for ii = 1:length(int)
73     plot(int(ii,1),int(ii,2),'om','linewidth',2);
74 end
75
76 plot(Fr1_array, L_over_y1_array,'m','linewidth',2);
77 %plot(Fr1, L_over_y1,'co','linewidth',2);
78
79 %print('103','-dpng','-r600');
80
81 end
82 end
83

```



### A.3.8 Function: $\frac{h_3}{D_1} = f(\text{Fr}_2)$ Reader

```

1 function [h3_over_D1_out] = func_h3_over_D1_from_Fr2(Fr2, plot_fig)
2
3 % --- set axis scaling -----
4
5 ori = [36 459];
6 x_end = 891;
7 y_top = 67;
8 y_top_unscaled = 4 ;
9 x_end_unscaled = 18;
10
11 scale_xx = (x_end - ori(1))/x_end_unscaled;
12 scale_yy = (ori(2)- y_top)/y_top_unscaled;
13
14 % --- record points for interpolation -----
15
16 int = [227 332 ;880 105 ] ;
17
18 % --- create matrix of interpolated values -----
19
20 [rows, ~] = size(int);
21 AA = ones(rows,rows);
22
23 for ii = 1:rows
24     for jj = 1:rows
25         AA(ii,jj) = int(ii,1)^(rows-jj);
26     end
27 end
28
29 CC = AA\int(:,2);
30
31 % -- create matrix of Cd values, from which the output Cd is interpolated -
32
33 N_plot = 1000;
34
35 Fr1_array = linspace(int(1,1),max(int(:,1)),N_plot);
36 h3_over_D1_array = zeros(N_plot);
37
38 for ii = 1:N_plot
39     for jj = 1:rows
40         h3_over_D1_array(ii) = h3_over_D1_array(ii) + ...
41             CC(jj)*Fr1_array(ii)^(rows-jj);
42     end
43 end
44
45 % --- interpolate initial output value, with Cd = 0.85 -----
46
47 Fr1_unscaled = Fr2; % accounting for chart notation
48 Fr1 = Fr1_unscaled*scale_xx + ori(1);
49
50 h3_over_D1= 0;
51
52 for jj = 1:rows
53     h3_over_D1 = h3_over_D1 + CC(jj)*Fr1^(rows-jj);
54 end
55
56 h3_over_D1_unscaled = y_top_unscaled - (h3_over_D1 - y_top)/scale_yy;
57 h3_over_D1_out = h3_over_D1_unscaled;
58
59 % --- plot results if requested -----
60 if plot_fig == 1
61
62     figure;
63     I = imread('fig18.png');
64     B = imrotate(I,0.00);
65
66     imshow(B); hold on
67     plot(ori(1),ori(2), 'or');
68     plot([ori(1) x_end],[ori(2) ori(2)], '-b', 'linewidth',2);
69     plot([ori(1) ori(1)],[y_top ori(2)], '-r', 'linewidth',2);
70
71     for ii = 1:length(int)
72         plot(int(ii,1),int(ii,2), 'om', 'linewidth',2);
73     end
74
75     plot(Fr1_array, h3_over_D1_array, 'm', 'linewidth',2);
76     plot(Fr1, h3_over_D1, 'co', 'linewidth',2);
77
78 end
79 end

```

### A.3.9 Function: $\frac{h_4}{D_1} = f(\text{Fr}_2)$ Reader

```

1 function [h4_over_D1_out] = func_h4_over_D1_from_Fr2(Fr2, plot_fig)
2
3 % --- set axis scaling -----
4
5 ori = [36 459];
6 x_end = 891;
7 y_top = 67;
8 y_top_unscaled = 4 ;
9 x_end_unscaled = 18;
10
11 scale_xx = (x_end - ori(1))/x_end_unscaled;
12 scale_yy = (ori(2)- y_top)/y_top_unscaled;
13
14 % --- record points for interpolation -----
15
16 int = [227 340 ;891 261 ] ;
17
18 % --- create matrix of interpolated values -----
19
20 [rows, ~] = size(int);
21 AA = ones(rows,rows);
22
23 for ii = 1:rows
24     for jj = 1:rows
25         AA(ii,jj) = int(ii,1)^(rows-jj);
26     end
27 end
28
29 CC = AA\int(:,2);
30
31 % -- create matrix of Cd values, from which the output Cd is interpolated -
32
33 N_plot = 1000;
34
35 Fr1_array = linspace(int(1,1),max(int(:,1)),N_plot);
36 h4_over_D1_array = zeros(N_plot);
37
38 for ii = 1:N_plot
39     for jj = 1:rows
40         h4_over_D1_array(ii) = h4_over_D1_array(ii) + ...
41             CC(jj)*Fr1_array(ii)^(rows-jj);
42     end
43 end
44
45 % --- interpolate initial output value, with Cd = 0.85 -----
46
47 Fr1_unscaled = Fr2; % accounting for chart notation
48 Fr1 = Fr1_unscaled*scale_xx + ori(1);
49
50 h4_over_D1= 0;
51
52 for jj = 1:rows
53     h4_over_D1 = h4_over_D1 + CC(jj)*Fr1^(rows-jj);
54 end
55
56 h4_over_D1_unscaled = y_top_unscaled - (h4_over_D1 - y_top)/scale_yy;
57 h4_over_D1_out = h4_over_D1_unscaled;
58
59 % --- plot results if requested -----
60 if plot_fig == 1
61
62     figure;
63     I = imread('fig18.png');
64     B = imrotate(I,0.00);
65
66     imshow(B); hold on;
67     plot(ori(1),ori(2), 'or');
68     plot([ori(1) x_end],[ori(2) ori(2)], '-b', 'linewidth',2);
69     plot([ori(1) ori(1)],[y_top ori(2)], '-r', 'linewidth',2);
70
71     for ii = 1:length(int)
72         plot(int(ii,1),int(ii,2), 'or', 'linewidth',2);
73     end
74
75     plot(Fr1_array, h4_over_D1_array, 'r', 'linewidth',2);
76     plot(Fr1, h4_over_D1, 'co', 'linewidth',2);
77
78     disp(h4_over_D1_out);
79
80 end
81 end

```